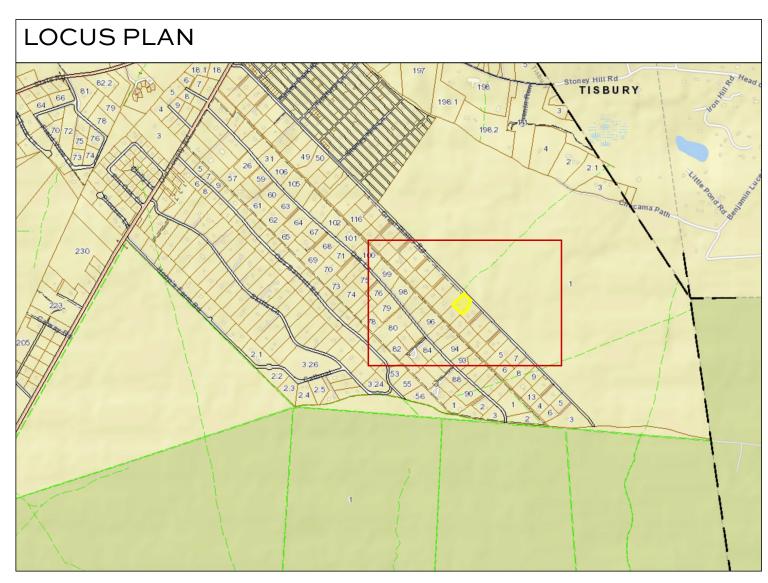
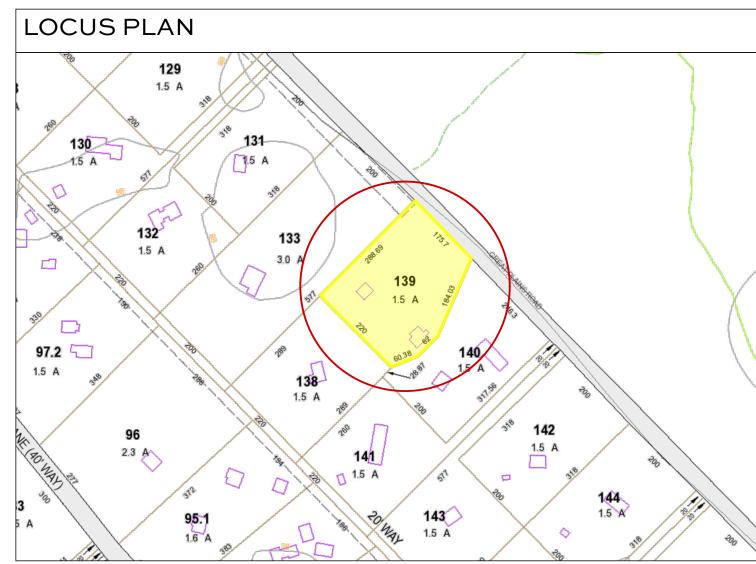


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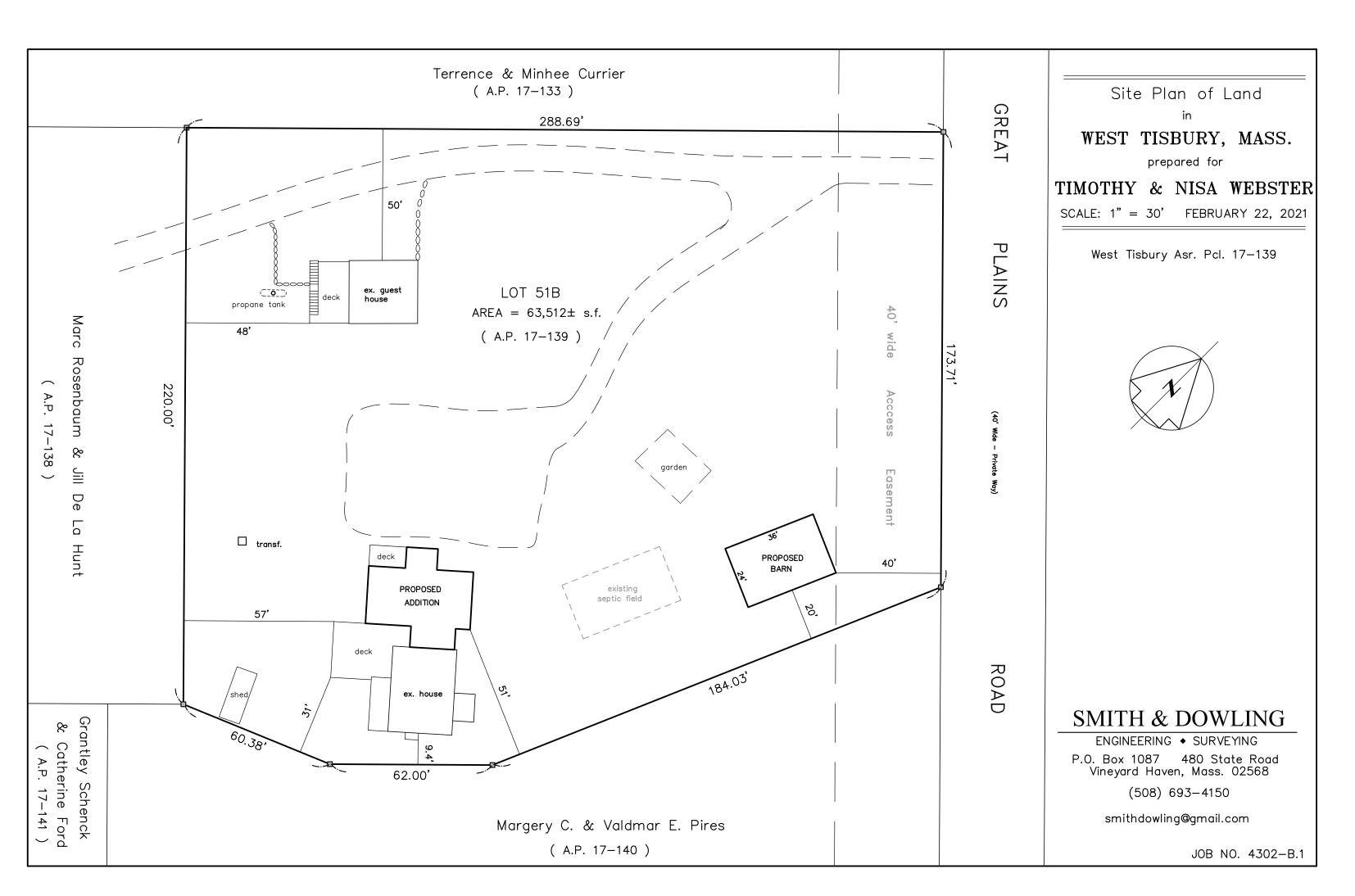


WESBTER HOUSE

233 GREAT PLAINS ROAD WEST TISBURY, MASSACHUSETTS

MAP17 - LOT139

PERMIT DRAWINGS





G001 COVER SHEET

O1 EXISTING/DEMO FLOOR PLANS 201 EXISTING/DEMO ELEVATIONS

A100 BASEMENT PLAN
A101 1ST FLOOR PLAN
A102 2ND FLOOR PLAN

A201 SOUTH + EAST ELEVATIONS
A202 NORTH + WEST ELEVATIONS
A301 BUILDING SECTION

501 DETAILS

S001 STRUCTURAL NOTES S100 FOUNDATION PLAN

1ST + 2ND FLOOR FRAMING PLANS

S102 ROOF FRAMING PLAN
S103 SHEAR WALL LAYOUT
S104 SHEAR WALL ELEVATIONS

S105 STRUCTURAL DETAILS

SQUARE FOOTAGE ABOVE GRADE (PER TOWN BY-LAW):

EXISTING NEW TOTAL
MAIN FLOOR 872 SF 1,112 SF 1,984 SF
SECOND FLOOR 332 SF 945 SF 1,277 SF
TOTAL: 3,261 SF

used for construction unless noted as such.

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Drawing Set Type and Issue Date:

PERMIT SET

30 MAR. 2021

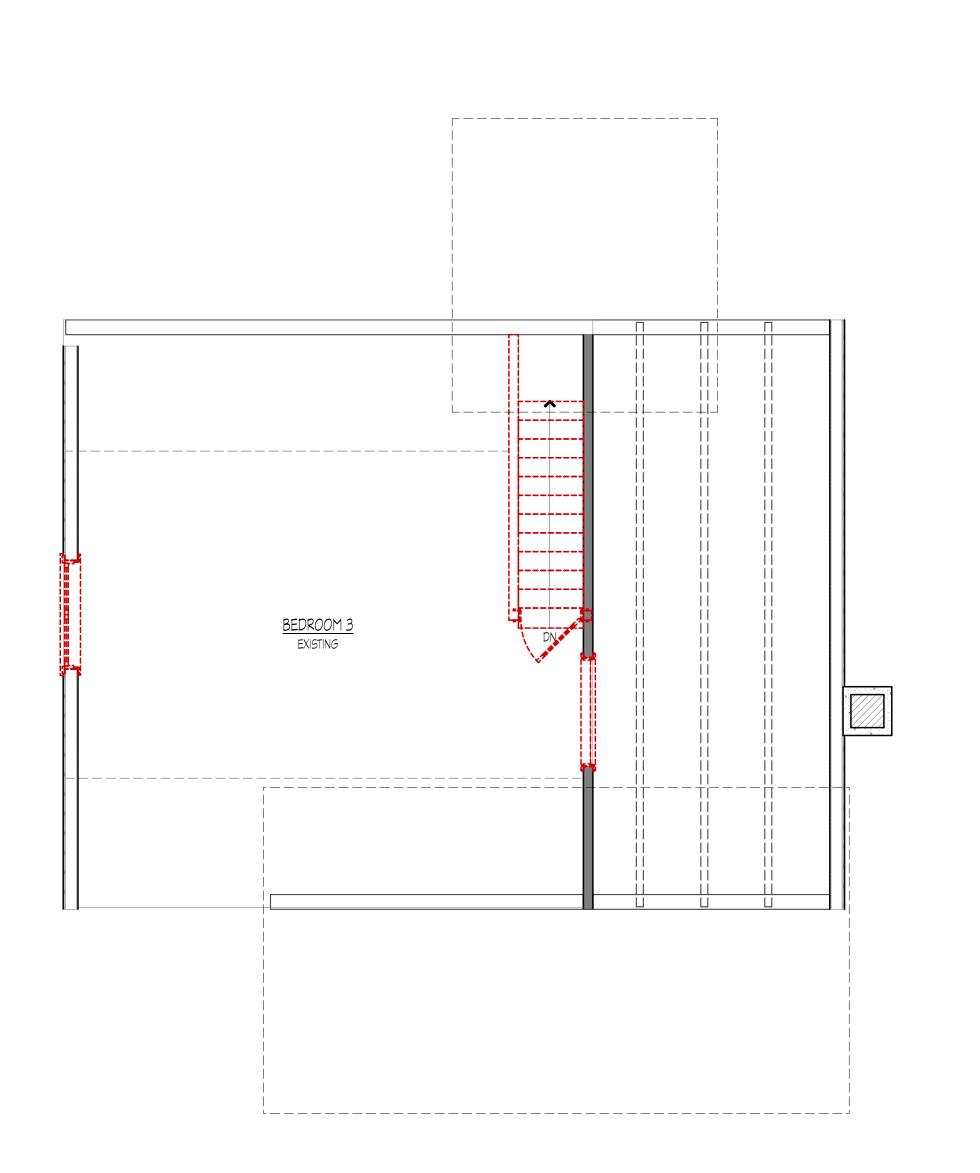
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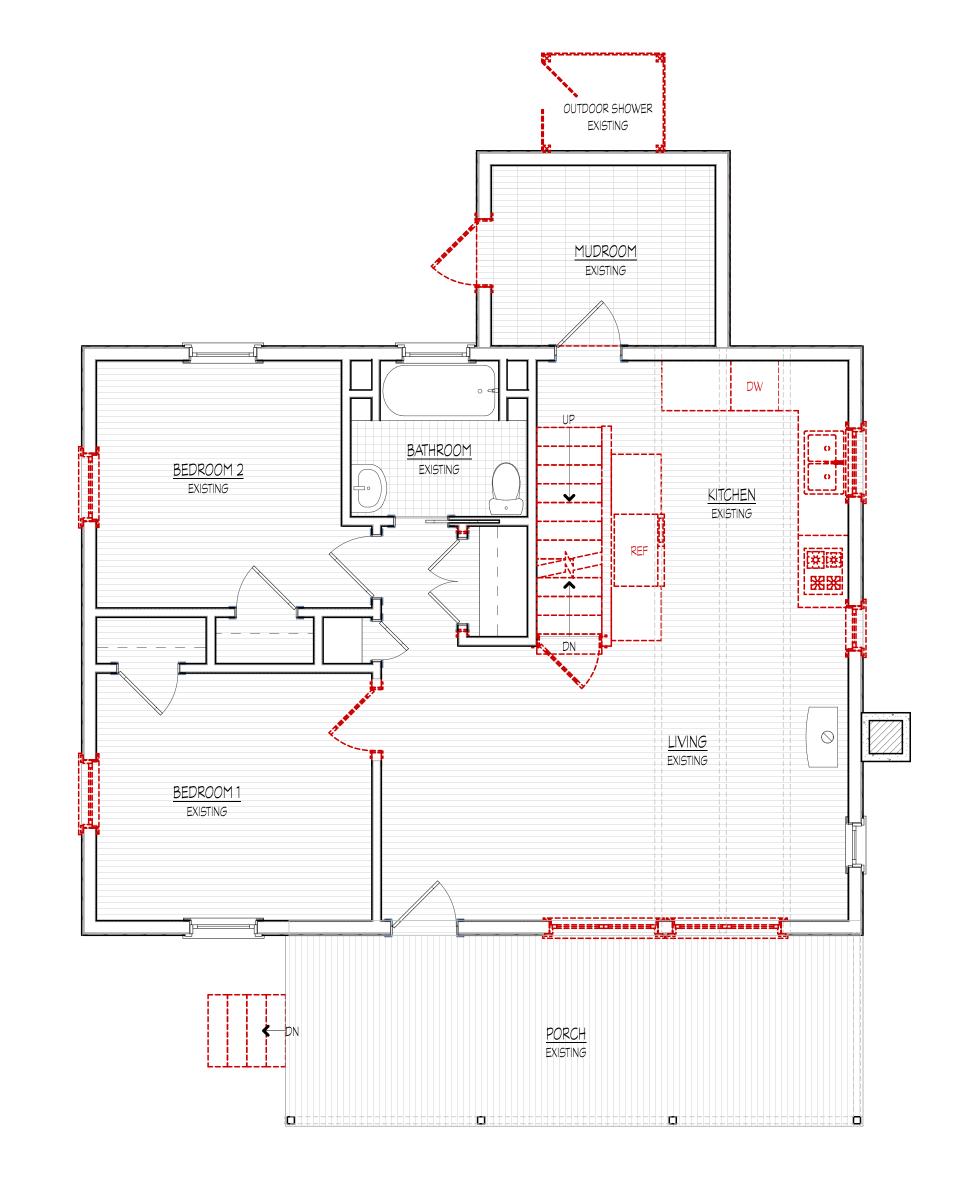
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G001





SECOND FLOOR DEMO PLAN

SCALE: 1/4" = 1'-0"

FIRST

SCALE: 1/4" = 1'-0"

FIRST FLOOR DEMO PLAN

SCALE: 1/4" = 1'-0"

0 2' 4'

EXISTING CONSTRUCTION TO BE REMOVED

EXISTING CONSTRUCTION TO REMAIN

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WEBSTER, 233 GREAT PLA

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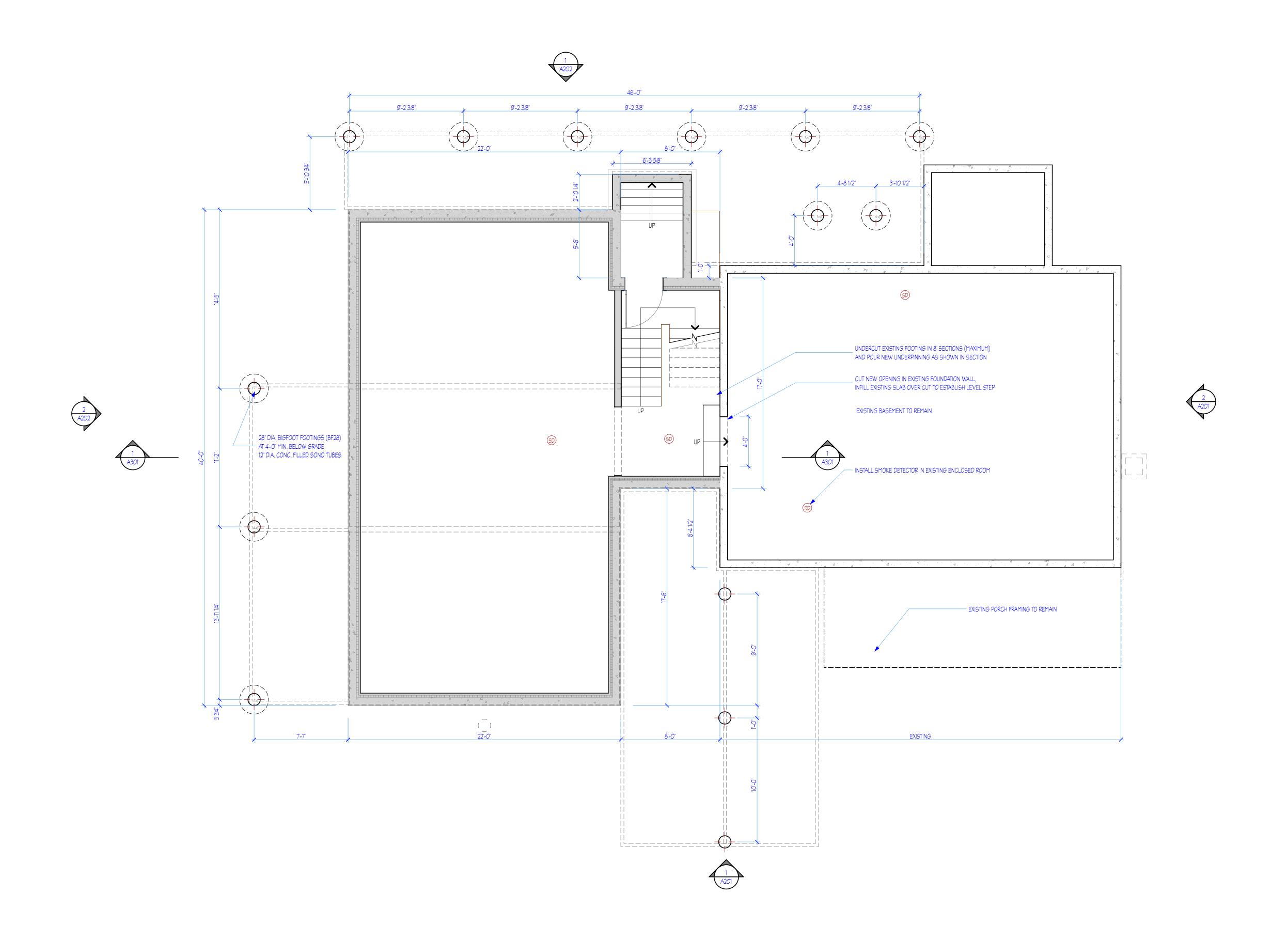
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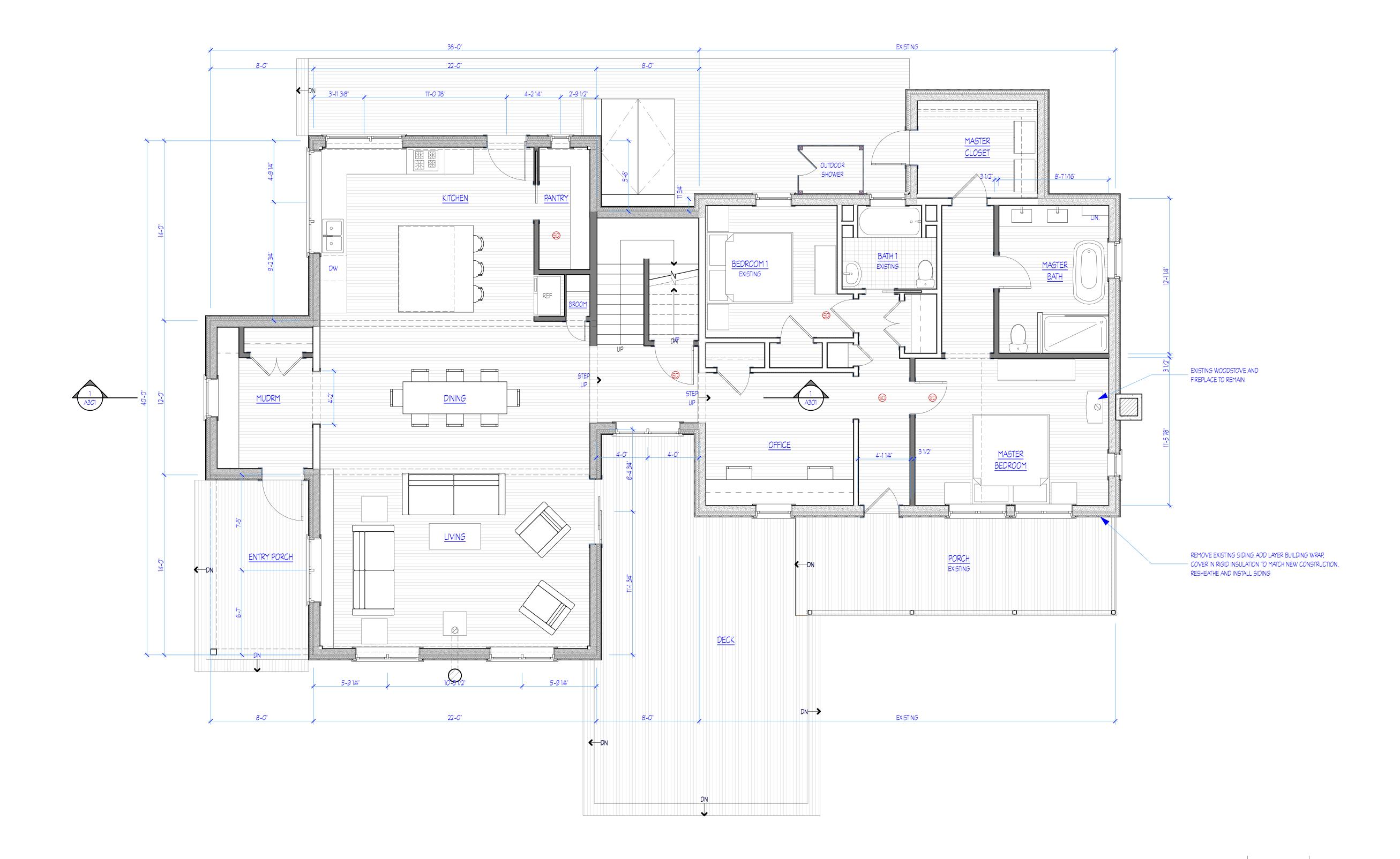
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17

TISBURY MA MAP

RD.



1st FLOOR
WEBSTER AD
233 GREAT PLAIN

NEW CONSTRUCTION

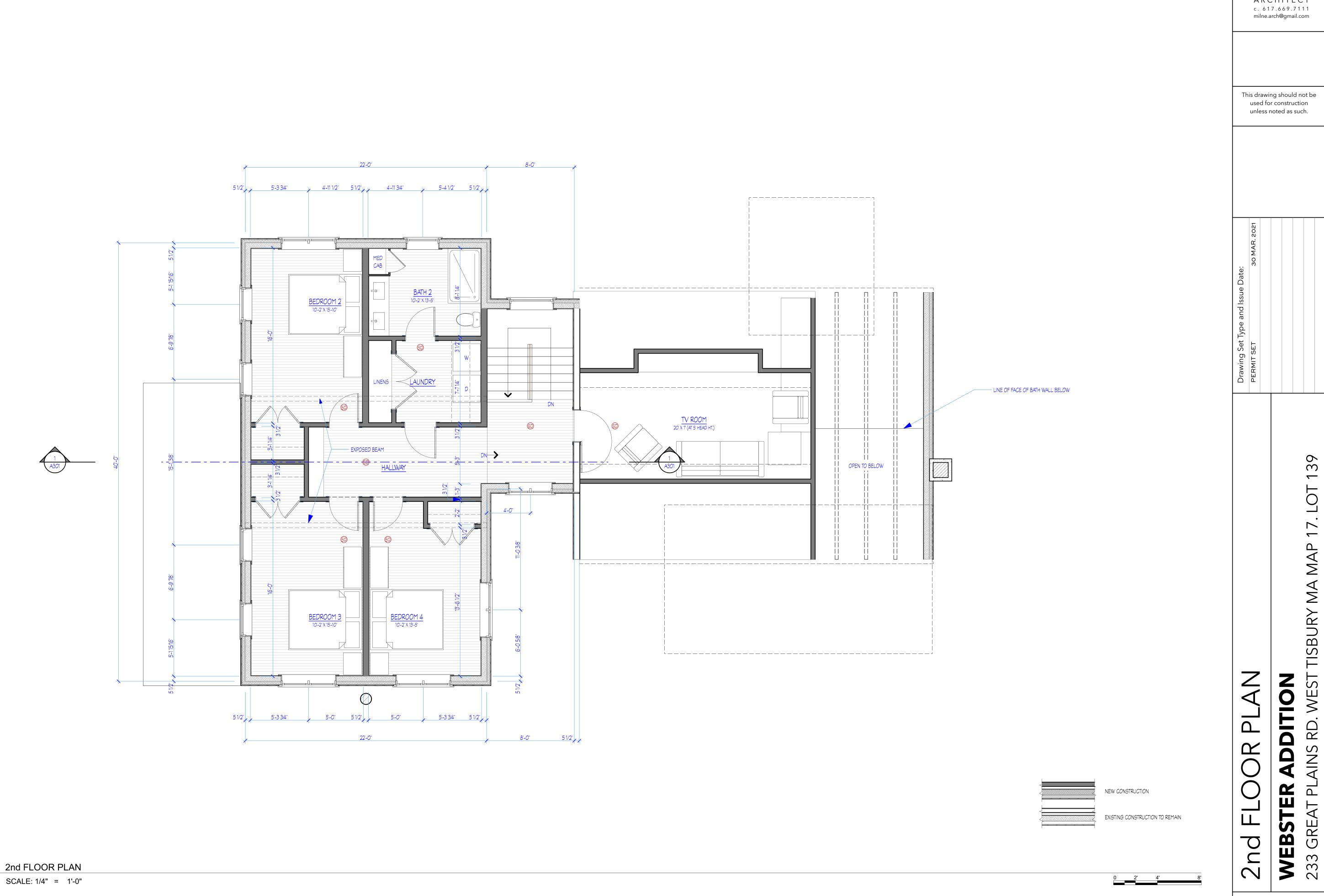
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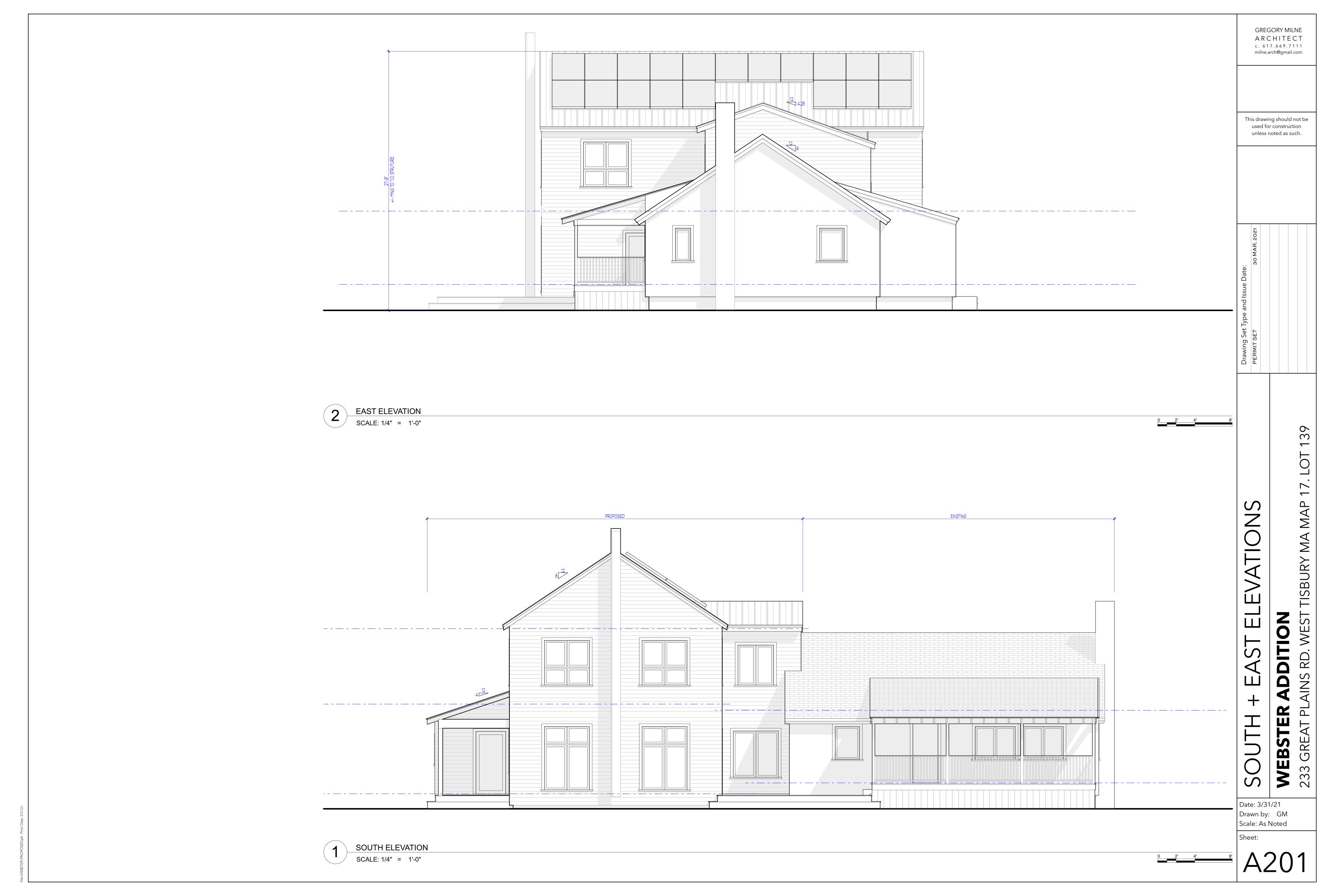


GREGORY MILNE ARCHITECT

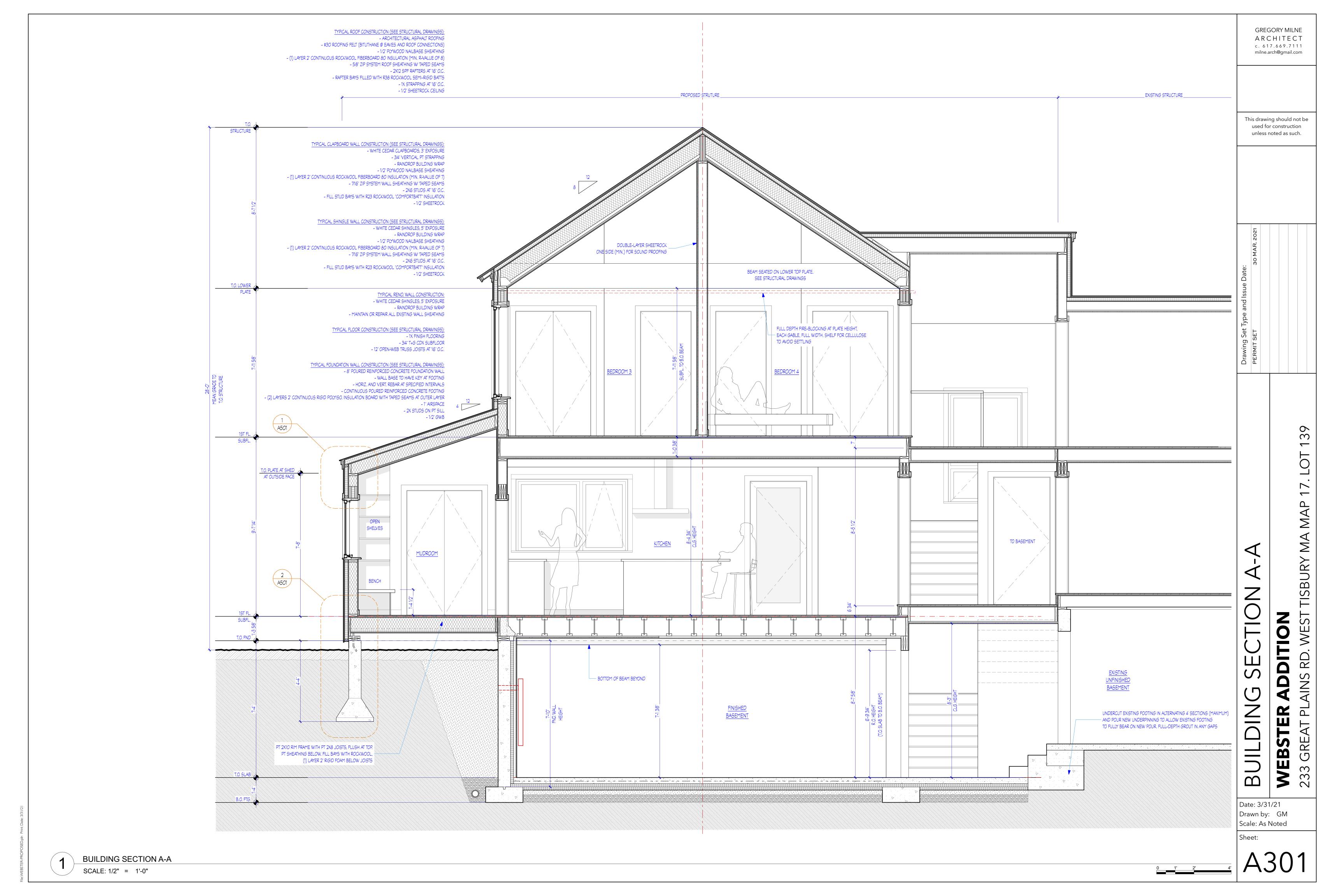
39 **TISBURY MA MAP**

Date: 3/31/21 Drawn by: GM Scale: As Noted

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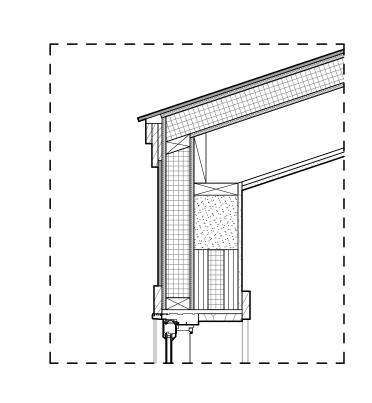
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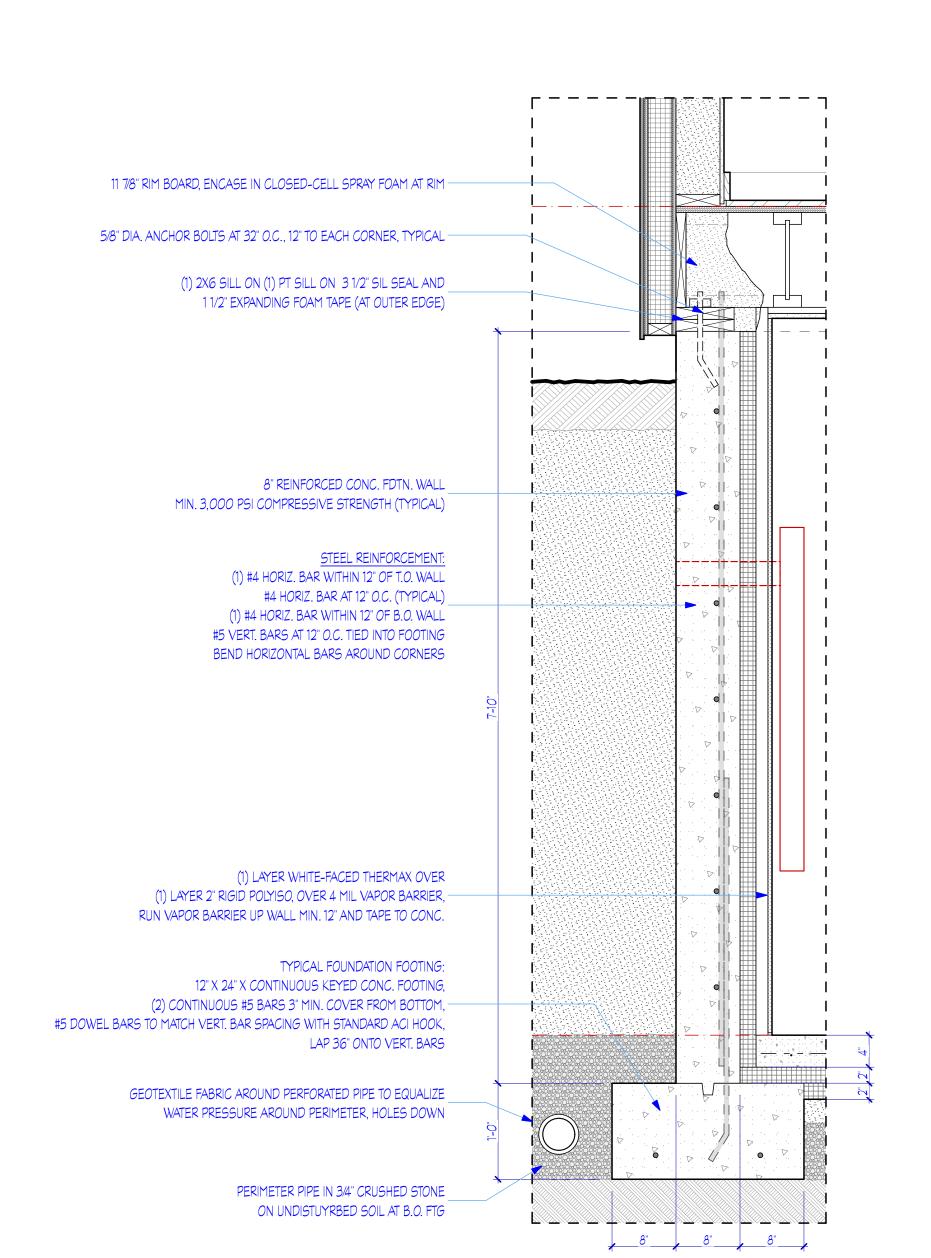
 \mathcal{C} MAP **TISBURY MA**

BUILDING GRE, **WEB**: 233 GR

Date: 3/31/21 Drawn by: GM Scale: As Noted



DETAIL AT MUDROOM EAVE



TYPICAL FOUNDATION DETAIL

GENERAL NOTES

GENERAL

- 1. REFER TO DETAILED PROJECT SPECIFICATIONS FOR GENERAL CONTRACT REQUIREMENTS AND DETAILED REQUIREMENTS FOR MATERIALS, WORKMANSHIP AND SHOP DRAWINGS.
- 2. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS AND TO APPROVED SHOP DRAWINGS FOR LOCATION AND DIMENSIONS OF CHASES, INSERTS, OPENINGS, SLEEVES, DEPRESSIONS AND REQUIREMENTS FOR ATTACHMENT OF FINISHES.
- 3. ALL DIMENSIONS OTHER THAN PURELY STRUCTURAL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE FIELD COORDINATED BY THE CONTRACTOR WITH THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS AND ANY INCONSISTENCIES REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.
- 4. THE STRUCTURAL DESIGN OF THE BUILDING IS BASED ON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS. NO PROVISIONS HAVE BEEN MADE FOR CONDITIONS OCCURRING DURING CONSTRUCTION. ANY FAILURE TO MAKE PROPER AND ADEQUATE PROVISIONS TO MAINTAIN STABILITY OF THE PARTIALLY COMPLETED STRUCTURE DURING CONSTRUCTION SHALL BE THE SOLE RISK
- 5. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS IN THE FIELD. NOTIFY THE ARCHITECT IMMEDIATELY, IN WRITING, OF ANY FIELD CONDITION UNCOVERED DURING CONSTRUCTION THAT IS NOT CONSISTENT WITH THE PLANS, THAT IS STRUCTURALLY INADEQUATE, OR THAT WILL IMPAIR ARCHITECTURAL LAYOUTS OR ATTACHMENTS OF FINISHES.
- 6. UNLESS OTHERWISE NOTED, DETAILS SHOWN ON ANY DRAWING ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
- 7. UNLESS OTHERWISE NOTED ALL FOOTINGS AND PIERS SHALL BE CENTERED UNDER SUPPORTED MEMBERS.
- 8. ALL FOUNDATION WALLS SHALL BE BRACED DURING THE OPERATIONS OF BACKFILLING AND TAMPING. BRACING SHALL BE LEFT IN POSITION UNTIL PERMANENT RESTRAINTS HAVE BEEN INSTALLED.
- 9. WHEREVER SLEEVES ARE INSERTED IN CONCRETE SLABS, BEAMS OR WALLS, THEY SHALL CONSIST OF STEEL, CAST IRON PIPE, OR
- 10. THE ARCHITECT WILL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES OF PROCEDURES OR CONSTRUCTION OR THE SAFETY PRECAUTIONS AND PROGRAMS INCIDENT THERETO, AND THE ARCHITECT WILL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO PERFORM THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

SHOP DRAWINGS AND SUBMITTALS

- 1. THE CONTRACTOR SHALL REVIEW AND APPROVE ALL SHOP DRAWING SUBMITTALS AND SHALL PROVIDE ANSWERS TO ALL FABRICATOR QUESTIONS REGARDING FIELD DIMENSIONS, ELEVATIONS, ETC. PRIOR TO SUBMITTAL TO THE ARCHITECT. THE CONTRACTOR SHALL ALSO ASSIGN EACH SUBMITTAL A UNIQUE NUMBER SO THAT IT MAY BE IDENTIFIED. SHOP DRAWINGS AND OTHER SUBMITTALS NOT BEARING THE CONTRACTOR'S APPROVAL STAMP, OR WHICH ARE NOT UNIQUELY NUMBERED, OR WHICH STILL HAVE FIELD DIMENSIONS, ELEVATIONS, ETC. MISSING WILL NOT BE REVIEWED AND WILL BE RETURNED NOT APPROVED.
- 2. UNLESS NOTED OTHERWISE IN THE PROJECT SPECIFICATIONS, THE ARCHITECT SHALL BE PROVIDED WITH TWO PAPER COPIES OF EACH SUBMITTAL. ONE COPY SHALL BE RETAINED BY THE ARCHITECT AND ONE COPY WILL BE RETURNED TO THE CONTRACTOR FOR REPRODUCTION AND DISTRIBUTION TO PARTIES
- 3. UNLESS NOTED OTHERWISE IN THE PROJECT SPECIFICATIONS, THE CONSTRUCTION SCHEDULE SHALL ALLOW THE ARCHITECT TEN WORKING DAYS TO REVIEW EACH SEPARATE SUBMITTAL. THE TIME THE SUBMITTAL SPENDS IN TRANSIT TO AND FROM THE ARCHITECT'S OFFICE SHALL NOT COUNT AS PART OF THE REVIEW PERIOD.

<u>DESIGN LOADS</u>

- 1. THE VARIOUS PORTIONS OF THE STRUCTURE ARE DESIGNED TO CARRY THE FOLLOWING LIVE LOADS: REFER TO LOAD TABLE ON
- 2. LATERAL LOADS: REFER TO LOAD TABLE ON SHEET S7.

DEMOLITION, SHORING AND UNDERPINNING WORK

- 1. BEFORE PROCEEDING WITH ANY DEMOLITION, THE AREAS MUST BE SURVEYED AND EVALUATED BY THE CONTRACTOR TO ENSURE THAT NO DAMAGE WILL BE MADE TO ANY STRUCTURE BEYOND THE DEMOLITION.
- 2. PROVIDE TEMPORARY SHORING AND BRACING FOR FLOORS, ROOFS, PIERS AND WALLS DURING DEMOLITION AND MAINTAIN THIS TEMPORARY CONSTRUCTION IN PLACE UNTIL THE NEW STRUCTURAL WORK IS COMPLETED AND TIED TO THE REMAINING EXISTING CONSTRUCTION. REMOVE DEMOLISHED ITEMS PROMPTLY FROM THE BUILDING. DO NOT OVERLOAD EXISTING FLOORS WITH
- 3. REMOVE AND RELOCATE AS REQUIRED UTILITIES CROSSING EXCAVATIONS AND NEW FOUNDATION WORK. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SUPPORT FOR ALL UTILITY LINES ADJACENT TO THE NEW FOUNDATION WORK.
- 4. PROTECT STREETS, SIDEWALKS, AND ADJACENT BUILDING FOUNDATIONS DURING EXCAVATIONS BY SHEET PILING, BRACING, SHORING, ETC. AS REQUIRED BY FIELD CONDITIONS. EXCAVATIONS AND SHORING SHALL BE INSPECTED BY A COMPETENT REGISTERED ENGINEER EMPLOYED BY THE CONTRACTOR. PROTECTION AGAINST SLIDES AND CAVE-INS SHALL BE INCREASED IF HE/SHE DEEMS
- 5. REMOVE ALL EXISTING FOUNDATIONS INTERFERING WITH THE NEW WORK. THE CONTRACTOR SHALL PROVIDE ALL INFORMATION PERTAINING TO EXISTING FOUNDATIONS SO THAT THE ARCHITECT CAN ASSESS ANY NECESSARY CHANGE TO THE FUTURE

EXCAVATIONS, FOUNDATIONS AND BACKFILL

- 1. REMOVE ALL FILL AND SOFT OR ORGANIC MATERIALS FROM UNDER OR ADJACENT TO ALL FOOTINGS AND FROM UNDER ALL SLABS ON GRADE.
- 2. EXTEND AND SLOPE SIDES OR SHORE, SHEET AND BRACE EXCAVATIONS AS REQUIRED TO ENSURE STABILITY AND SAFETY AT ALL
- 3. PUMP THE EXCAVATION TO REMOVE SURFACE AND GROUNDWATER, TO PERMIT FINISHING OF THE EXCAVATION AND PLACING OF FOUNDATIONS IN THE DRY. NO FOOTING SHALL BE PLACED IN WATER.
- 4. THE ARCHITECT ASSUMES NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED IN THE DOCUMENTS. THIS DATA IS INCLUDED ONLY TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION, AND REPRESENTS CONDITIONS ONLY AT THOSE SPECIFIC LOCATIONS AT THE PARTICULAR TIME THEY WERE MADE.

2000 POUNDS PER SQUARE FOOT, OR SHALL BEAR ON COMPACTED FILL AS DESCRIBED IN NOTE #10 BELOW.

- 5. ALL FOOTINGS SHALL BE CARRIED DOWN TO 4-INCHES INTO THE UNDISTURBED LAYER HAVING A MINIMUM BEARING CAPACITY OF
- 6. BEARING MATERIAL, DESIGN BEARING PRESSURE AND FOOTING ELEVATIONS INDICATED ON THE DRAWINGS ARE ASSUMED. ELEVATIONS GIVEN ARE MINIMUM IN DEPTH AND ARE NOT TO BE CONSTRUED AS LIMITING IN ANY WAY THE AMOUNT OF EXCAVATION REQUIRED TO REACH A SPECIFIED BEARING, IF UPON EXCAVATION TO THE LEVELS SHOWN, ACCEPTABLE BEARING MATERIAL IS NOT ENCOUNTERED, THE FOOTING SHALL BE LOWERED OR INCREASED IN SIZE AT THE DIRECTION OF THE ARCHITECT.
- 7. TYPICAL FOOTING EXCAVATIONS WILL BE INSPECTED BY THE ARCHITECT BEFORE THE FOOTINGS ARE CAST IN ORDER TO CONFIRM THAT THE FOUNDATION MATERIAL IS ADEQUATE TO SUSTAIN THE DESIGN BEARING PRESSURE.
- 8. ALL EXTERIOR CONSTRUCTION SHALL BE CARRIED DOWN A MINIMUM OF FOUR (4) FEET BELOW FINISHED EXTERIOR GRADE, UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
- 9. MATERIAL ADJACENT TO AND BELOW THE FOOTINGS SHALL BE KEPT FROM FREEZING AT ALL TIMES. IF ANY MATERIAL IS FOUND TO BE FROZEN IT SHALL BE REMOVED AND REPLACED WITH CONCRETE. ANY FROZEN MATERIAL FOUND BELOW A SLAB ON GRADE SHALL BE REMOVED AND REPLACED WITH GRANULAR MATERIAL COMPACTED TO 95% OF MAXIMUM DENSITY, AS DESCRIBED IN NOTE
- 10. ALL STRUCTURAL FILL SHALL BE PLACED IN LAYERS NOT MORE THAN 8-INCHES IN LOOSE DEPTH AND COMPACTED TO THE FOLLOWING PERCENTAGES OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM TEST METHOD D1557 (MODIFIED PROCTOR): 95% BENEATH FOOTINGS, 95% BENEATH SLABS ON GRADE. THE ENTIRE FILLING AND COMPACTING OPERATIONS ARE TO BE WITNESSED BY A TESTING LABORATORY RETAINED BY THE OWNER. CLEAN EXCAVATED MATERIALS MEETING THE SPECIFICATIONS MAY BE USED AS BACKFILL. STRUCTURAL FILL SHALL CONFORM TO THE FOLLOWING GRADATION:

PERCENT PASSING 3.5-INCH 1.5-INCH 55%-100% 1/4-INCH 25%-60% 15%-45% 5%-25% 0%-5%

11. INSTALL FOUNDATION WALL AND UNDER-FLOOR DRAINAGE AS SHOWN ON THE DRAWINGS.

12. SEE ARCHITECT'S DRAWINGS AND SPECIFICATIONS FOR PERIMETER INSULATION AND EXTERIOR WATERPROOFING.

<u>CONCRETE</u>

- 1. ALL CONCRETE SHALL CONFORM TO THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318), THE STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE IN BUILDINGS (ACI 301) AND LOCAL BUILDING CODES. ALL CONCRETE WORK SHALL BE AS SPECIFIED AND RECOMMENDED BY ACI FIELD REFERENCE MANUAL SP-15.
- 2. CONCRETE COMPRESSIVE DESIGN STRENGTHS AND MIX PROPORTIONS SHALL BE AS OUTLINED BELOW. MIX PROPORTIONS AND DSIGNS SHALL BE SUBMITTED FOR APPROVAL. LIMIT MAXIMUM WATER-SOLUBLE CHLORIDE ION CONTENT IN CONCRTE BY WEIGHT OF CEMENT FOR CAST-IN-PLACE CONCRETE TO 0.3 FOR INTERIOR CONCRETE AND 0.15 FOR EXTERIOR CONCRETE.

CONCRETE USAGE CONCRETE CLASS COMPRESSIVE STRENGTH MAX. W/CM RATIO FOOTINGS F0, S0, W0, C1 3,000 PSI AT 28-DAYS 5% +/- 1.5% WALLS AND PIFRS F1, S0, W0, C1 3.500 PSI AT 28-DAYS 0.55 5% +/- 1.5% INTERIOR SLABS ON GRADE 3,500 PSI AT 28-DAYS F0, S0, W0, C0

- 3. ALL REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO REQUIREMENTS OF ASTM SPECIFICATION A615. GRADE 60.
- 4. ALL WELDED WIRE REINFORCING (WWR) SHALL BE PLAIN, COLD DRAWN, ELECTRICALLY WELDED REINFORCING CONFORMING TO THE REQUIREMENTS OF ASTM A185. SUPPLY WELDED WIRE REINFORCING IN FLAT SHEETS, LAP SHEETS 1-1/2 MESH.

- 5. ALL REINFORCING MARKED CONTINUOUS (CONT.) SHALL HAVE LAPS CONFORMING TO THE REQUIREMENTS OF ACI 318 AT SPLICES AND CORNERS AND SHALL BE HOOKED OR EXTENDED A MINIMUM OF 40 BAR DIAMETERS AT NON-CONTINUOUS ENDS. TOP BARS SHALL BE LAPPED AT MID SPAN; BOTTOM BARS SHALL BE LAPPED AT SUPPORTS.
- 6. REINFORCING SHALL BE SECURELY TIED IN ITS PROPER PLACE BEFORE AND DURING POURING OPERATIONS USING APPROVED CHAIRS AND SPACERS AS REQUIRED.
- 7. SLABS ON GRADE SHALL BE PLACED ON A 6-INCH THICK (MINIMUM) LAYER OF 95% COMPACTED GRAVEL, WITH CONSTRUCTION OR CONTROL JOINTS AT A MAXIMUM SPACING OF 15-FEET, AS DETAILED ON THE DRAWINGS. PROVIDE 6 MIL. CONTINUOUS VAPOR
- 8. ALL REINFORCEMENT SHALL BE CONTINUOUS ACROSS CONSTRUCTION JOINTS.
- 9. FOUNDATION WALLS SHALL BE POURED IN MAXIMUM LENGTHS OF 40-FEET, WITH KEYED AND DOWELED JOINTS. NO HORIZONTAL JOINTS SHALL BE PERMITTED EXCEPT AS SHOWN ON THE DRAWINGS.
- 10. AT ALL OPENINGS LARGER THAN TWO (2) FEET IN CONCRETE WALLS, UNLESS INDICATED OTHERWISE, PROVIDE #6 BARS EACH FACE ON EACH SIDE OF OPENING, EXTENDING A MINIMUM OF TWO (2) FEET BEYOND THE EDGE OF THE OPENING.
- 11. THE CONCRETE CONTRACTOR SHALL INSTALL OR GIVE OTHER TRADES AMPLE OPPORTUNITY TO INSTALL ALL ANCHORS, BOLTS, PLATES, NAILS, SLOTS, CHASES PIPE SLEEVES ETC., AS REQUIRED BY THESE TRADES. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE SETTING SCREEDS AND FORMS.
- 12. PROVIDE CLEARANCES FROM FACES OF CONCRETE TO REINFORCEMENT AS FOLLOWS:
 - CONCRETE CAST AGAINST EARTH: 3" (ALL BARS) CONCRETE EXPOSED TO EARTH OR WEATHER: $1\frac{1}{2}$ " (#5 AND SMALLER); 2" (#6 AND LARGER) CONCRETE NOT EXPOSED TO EARTH OR WEATHER: SLABS AND WALLS: $\frac{3}{4}$ " (#11 AND SMALLER); PIERS: $1\frac{1}{2}$ " (MAIN STEEL AND TIES)
- 13. DO NOT CUT OR DISPLACE REINFORCING STEEL TO ACCOMMODATE INSTALLATION OF EMBEDDED ITEMS UNLESS APPROVED BY THE ARCHITECT. COORDINATE INSTALLATION OF SLEEVES, PIPES AND CONDUIT WITH THE PLACING OF REINFORCING STEEL TO ENSURE
- 14. NOTIFY THE ARCHITECT OF ANY CONCRETE PLACEMENT AT LEAST 24-HOURS IN ADVANCE, TO ALLOW INSPECTION OF THE REINFORCING STEEL. DO NOT PLACE CONCRETE UNTIL THIS INSPECTION HAS BEEN MADE OR WAIVED BY THE ARCHITECT.

- 1. ALL WOOD CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION BY THE NATIONAL FOREST PRODUCTS ASSOCIATION AND TO LOCAL BUILDING CODES.
- 2. ALL WOOD FOR FRAMING SHALL BE KILN-DRIED WITH A MAXIMUM MOISTURE CONTENT OF 19% AT THE TIME OF DRESSING.
- 3. ALL WOOD MEMBERS 2x6 AND LARGER SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES $\begin{array}{lll} \mathsf{Fb} &=& \mathsf{875} \; \mathsf{PSI} \\ \mathsf{Fv} &=& \mathsf{135} \; \mathsf{PSI} \end{array}$
 - Fc = 1150 PSI $E = 1.4x10^6 PSI$
- 4. ALL WOOD MEMBERS NOTED AS 4x6 SOUTHERN PINE SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES: Fb = 1350 PSIFv = 175 PSIFc = 1550 PSI
- 5. ALL WOOD MEMBERS NOTED AS 6x6 SOUTHERN PINE SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES: Fb = 1350 PSI
 - Fv = 165 PSIFc = 825 PSI

 $E = 1.5 \times 10^6 PSI$

 $E = 1.6 \times 10^6 PSI$

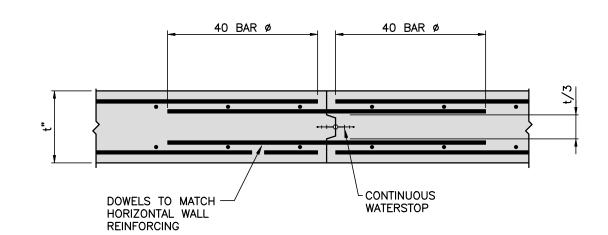
- 6. ALL WOOD MEMBERS NOTED AS PRESSURE TREATED FRAMING OR SILLS SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL
- PROPERTIES: Fb = 1350 PSI

 $E = 1.6 \times 10^6 PSI$

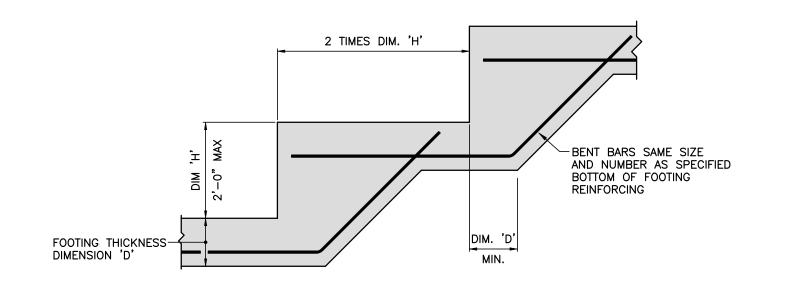
- Fv = 175 PSIFc = 1550 PSI
- 7. WOOD MEMBERS USED IN INTERIOR NON-LOAD BEARING PARTITIONS MAY BE STUD GRADE OF ANY COMMERCIALLY AVAILABLE
- 8. ALL WOOD STUD BEARING WALLS, EXTERIOR WALLS, AND WALLS GREATER THAN TEN (10) FEET IN HEIGHT SHALL BE 2x6 AT 16-INCHES ON CENTER, UNLESS OTHERWISE INDICATED. IN ALL BEARING WALLS PROVIDÉ A SINGLE BOTTOM SHOE, DOUBLE TOP PLATE AND SOLID WALL BRIDGING AT A MAXIMUM VERTICAL SPACING OF 48-INCHES.
- 9. ALL WALL STUDS SHALL BE LOCATED DIRECTLY UNDER FLOOR JOISTS/TRUSSES. PROVIDE DOUBLE STUDS ON EACH SIDE OF ALL OPENINGS, AND ADDITIONAL JACK STUDS TO SUPPORT HEADER BEAMS. FORM CORNERS WITH A MINIMUM OF THREE (3) STUDS SPIKED TOGETHER.
- 10. CONTINUITY IN FRAMING SHALL BE PROVIDED AT ALL BEARING WALLS IN ORDER TO TRANSFER THE LOADS TO THE FOUNDATION OR OTHER FRAMING, FULL DEPTH BLOCKING SHALL BE USED IN THE FLOOR FRAMING UNDER WOOD POSTS, BEARING WALLS, AND BUILT-UP STUDS TO PROVIDE FULL BEARING THROUGH FRAMING. DOUBLE JOISTS/TRUSSES SHALL BE USED UNDER ALL PARTITIONS RUNNING PARALLEL TO JOIST SPAN.
- 11. ALL WOOD HEADERS AT BEARING WALLS SHALL BE A MINIMUM OF (2) 2x8 UNLESS OTHERWISE SHOWN ON PLANS. WOOD MEMBERS USED FOR HEADERS OR BUILT-UP BEAMS SHALL NOT HAVE CHECKS OR SPLITS LONGER THAN THE WIDE FACE WIDTH.
- 12. ALL MEMBER TO MEMBER CONNECTIONS SHALL BE MADE WITH JOIST OR BEAM HANGERS, AND SHEET METAL POST BASES AND CAPS AS APPROPRIATE. JOIST HANGERS, FRAMING CLIPS AND OTHER HARDWARE SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY OR APPROVED EQUAL.
- 13. REFER TO THE MASSACHUSETTS STATE BUILDING CODE NAILING SCHEDULE FOR NAILING AND BOLTING NOT OTHERWISE SPECIFIED ON THE DRAWINGS.
- 14. DO NOT NOTCH THE TOPS OR BOTTOMS OF JOISTS IN THE MIDDLE THIRD OF THE SPAN. NOTCHES IN THE END THIRDS SHALL NOT EXCEED 1/6 OF THE JOIST DEPTH. HOLES DRILLED IN JOISTS SHALL NOT EXCEED 1/3 OF THE JOIST DEPTH AND SHALL NOT BE LOCATED WITHIN 2-INCHES OF THE TOP OR BOTTOM OF THE JOIST.
- 15. WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER FOR THE LOADS NOTED IN THE LOAD TABLE. LOADS SHALL BE APPLIED IN ANY AND ALL OF THE COMBINATIONS LISTED IN THE 2015 IBC CODE, WITH A TOTAL DEFLECTION NOT TO EXCEED SPAN/240. THE DESIGN OF TRUSSES SHALL CONFORM TO THE REQUIREMENTS OF THE DESIGN SPECIFICATIONS FOR LIGHT METAL PLATÉ CONNECTED WOOD TRUSSES PUBLISHED BY THE TRUSS PLATE INSTITUTE, BE STAMPED BY A REGISTERED PROFESSIONAL ENGINEER, AND BE SUBMITTED FOR APPROVAL PRIOR TO FABRICATION. THE MANUFACTURER SHALL DESIGN, SHOW, AND SUPPLY SPECIAL BEARINGS, UPLIFT ANCHORS, TRUSS TO TRUSS HANGERS AND LATERAL BRACING AS REQUIRED. TRUSS SHOP DRAWINGS SHOWING ANY ELEMENT OF A TRUSS THAT IS NOT COMPATIBLE WITH THE STRUCTURE AS SHOWN IN THE DRAWINGS WILL BE REJECTED. THE TRUSSES SHALL NOT RELY UPON HORIZONTAL RIGIDITY OF THE SURROUNDING STRUCTURE FOR STABILITY, I.E.: MODEL THE TRUSS WITH ONE SUPPORT RESTRAINED IN THE VERTICAL AND HORIZONTAL DIRECTION, THE OTHER SUPPORT RESTRAINED IN THE VERTICAL DIRECTION ONLY.
- 16. PLYWOOD FOR FLOOR SHEATHING SHALL BE APA GRADE-TRADE MARK 3/4-INCH TONGUE AND GROOVE COMBINED SUBFLOOR-UNDERLAYMENT GRADE PLYWOOD WITH EXTERIOR GLUE, SPECIES GROUP 1, 2, OR 3. LAY THE PLYWOOD SHEETS WITH THE FACE GRAIN PERPENDICULAR TO SUPPORT AND APPLY A 3/8-INCH DIAMETER BEAD OF CONSTRUCTION ADHESIVE TO THE TOP OF EACH JOIST/TRUSS AND TO THE TONGUE OF EACH ADJACENT PLYWOOD PANEL. ATTACH PLYWOOD TO JOIST WITH 8d DEFORMED SHANK NAILS (OR 8d COMMON WIRE NAILS) SPACED 6-INCHES ON CENTER AT ALL PANEL EDGES AND 12-INCHES ON CENTER AT ALL INTERMEDIATE SUPPORTS.
- 17. PLYWOOD FOR ROOF SHEATHING SHALL BE APA GRADE-TRADE MARK 5/8-INCH STANDARD 32/16 WITH EXTERIOR GLUE, LAID WITH THE FACE GRAIN PERPENDICULAR TO SUPPORT. PROVIDE 2x4 SOLID BLOCKING AT ALL PANEL EDGES BETWEEN FRAMING MEMBERS, AS NOTED ON DRAWINGS. PLYWOOD SHALL BE NAILED WITH 10d NAILS AT 6-INCHES ON CENTER AT ALL PANEL EDGES AND 12-INCHES ON CENTER AT INTERMEDIATE SUPPORTS.
- 18. PLYWOOD FOR SHEAR WALLS SHALL BE C-D GRADE 1/2-INCH FOR EXTERIOR WALLS. PROVIDE 2x4 SOLID BLOCKING AT ALL PANEL EDGES BETWEEN FRAMING MEMBERS. REFER TO SHEARWALL SCHEDULE ON SHEET S4 FOR NAILING AND HOLDOWN REQUIREMENTS.
- 19. PRESERVATIVE PRESSURE-TREAT WOOD WITH CHROMATED COPPER ARSENITE, TO A RETENTION OF 0.75 LB/CU.FT. FOR WOOD EXPOSED TO EARTH OR WEATHER, AND 0.35 LB/CU.FT. FOR WOOD IN CONTACT WITH CONCRETE, MASONRY OR ROOFING.
- 20. ALL HARDWARE AND FASTENERS EXPOSED TO WEATHER OR IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE GALVANIZED OR STAINLESS STEEL, AS RECOMMENDED BY THE HARDWARE OR FASTENER MANUFACTURER.

STRUCTURAL COMPOSITE LUMBER

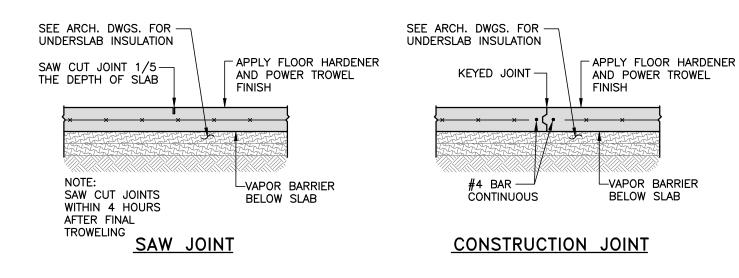
- 1. LAMINATED VENEER LUMBER (LVL) OR MICROLLAM (ML) MEMBERS SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES: Fb = 2600 PSI Fv = 285 PSI $E = 2.0X10^6 PSI$
- 2. PARALLEL STRAND LUMBER (PSL) OR PARALLAM (PL) MEMBERS SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES: Fb = 2400 PSIFv = 190 PSI $E = 1.8X10^6 PSI$
- 3. MULTIPLE MEMBERS SHALL BE NAILED TOGETHER WITH A MINIMUM OF THREE (3) 16D NAILS PER FOOT, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- 4. PARALLEL STRAND LUMBER (PSL) IS AN ACCEPTABLE SUBSTITUTE FOR LVL OR ML.
- 5. LVL AND PSL MEMBERS SHALL BE PROTECTED FROM THE EFFECTS OF MOISTURE DURING STORAGE.



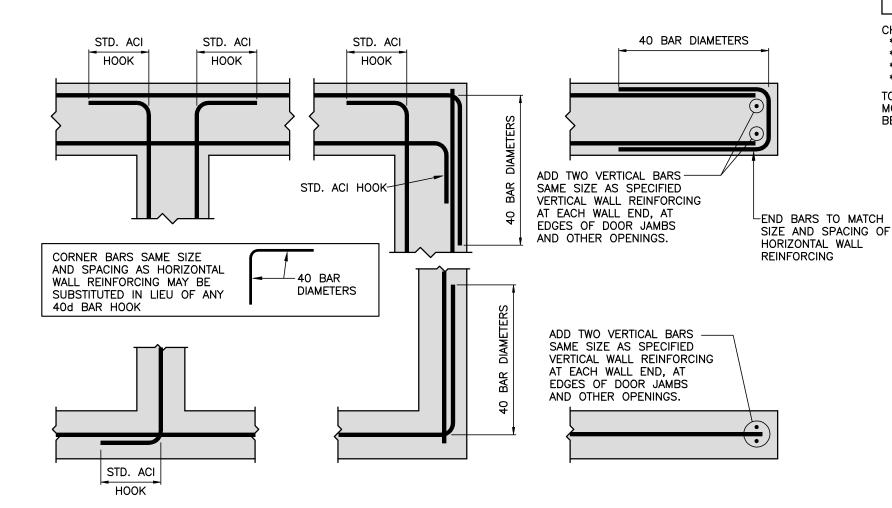
TYPICAL WALL CONSTRUCTION JOINT



TYPICAL STEP FOOTING DETAIL



TYPICAL SLAB ON GRADE JOINT DETAILS



TYPICAL REINFORCING DETAILS IN WALLS AND FOOTINGS

IBC 2015 EDITION RISK CATEGORY II - ALL OTHER STRUCTURES

1606	BASIC		
DEAD LOAD	UNIFORM		
ROOF	21 PSF		
FLOOR	15 PSF	·	

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1607	BASIC	REDUCED	POINT
FLOOR LIVE LOAD	UNIFORM	UNIFORM	
RESIDENTIAL	40 PSF		
EXTERIOR DECKS	60 PSF		
EXTENSIV BEONS	1 55 1 51		

1607.12	
ROOF LIVE LOAD	SNOW LOAD GOVERNS

1608		
ROOF SNOW LOAD		
GROUND SNOW	Pg	25 PSF
IMPORTANCE FACTOR	ls	1.00
THERMAL FACTOR	Ct	1.1
EXPOSURE FACTOR	Се	1.0
FLAT ROOF SNOW	Pf	25 PSF
DRIFT SURCHARGE	Pd	26.2 PSF
DRIFT WIDTH	w	6.1 FEET

1609						
WIND DESIGN DATA						
BASIC WIND SPEED		٧	140 MPH	3 SEC		
IMPORTANCE FACTOR	R	lw	1.00			
WIND EXPOSURE		SHEL	TERED	В		
DESIGN METHOD		METHOD 2 - ANALYTICAL				
INTERNAL COEFF		GCpi	±0.18			
COMPONENTS	R	ZONE 1	32.3 PSF	-35.3 F		
AND CLADDING	00F	ZONE 2	32.3 PSF	-41.2 F		
		ZONE 3	32.3 PSF	-41.2 F		
FOR 10 SO FOOT	W	ZONE 4	35.3 PSF	-38.2 F		
FOR 10 SQ FOOT TRIBUTARY AREA	L	ZONE 5	35.3 PSF	-47.2 F		

1613			
EARTHQUAKE			
IMPORTANCE FACTOR		le=	1.00
MAPPED RESPONSE AC	CCELER.	Ss=0.142	S1=0.0
SPECTRAL RESPONSE	COEFF.	Sds=0.151	Sd1=0.
SITE CLASS	D		
SEISMIC DESIGN CATE	GORY	В	
LATERAL FORCE SYSTE	:M	TABLE 12	.2-1: /
COEFF. & FACTORS	R = 6.5		Cd =
ANALYSIS PROCEDURE:	LATERAL FO	ORCE	
RESPONSE COEFFICIEN	Cs	0.023	
DESIGN BASE SHEAR	V = 1.	65 KIP:	

NOT APPLICABLE

FLOOD LOAD

REINFORCING SPLICE LENGTHS										
	f' _c =3,0	000 PSI	f' _c =4,0	000 PSI	f' _c =5,000 PSI					
BAR	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS				
#3	28"	22"	25"	19"	22"	17"				
#4	38"	29"	33"	25"	29"	23"				
#5	47"	36"	41"	31"	36"	28"				
#6	56"	43"	49"	37"	43"	34"				
#7	82"	63"	70"	54"	63"	48"				
#8	93"	72"	80"	62"	71"	55"				
#9	105"	81"	90"	69"	80"	62"				
#10	119"	91"	101"	78"	91"	70 "				
#11	132"	101"	112"	87"	100"	77"				

CHART BASED ON THE FOLLOWING: * REINFORCING BARS GRADE 60 (Fy=60 KSI) * BAR SPACING 2 BAR DIAMETERS MINIMUM * CLEAR COVER 1 BAR DIAMETER MINIMUM

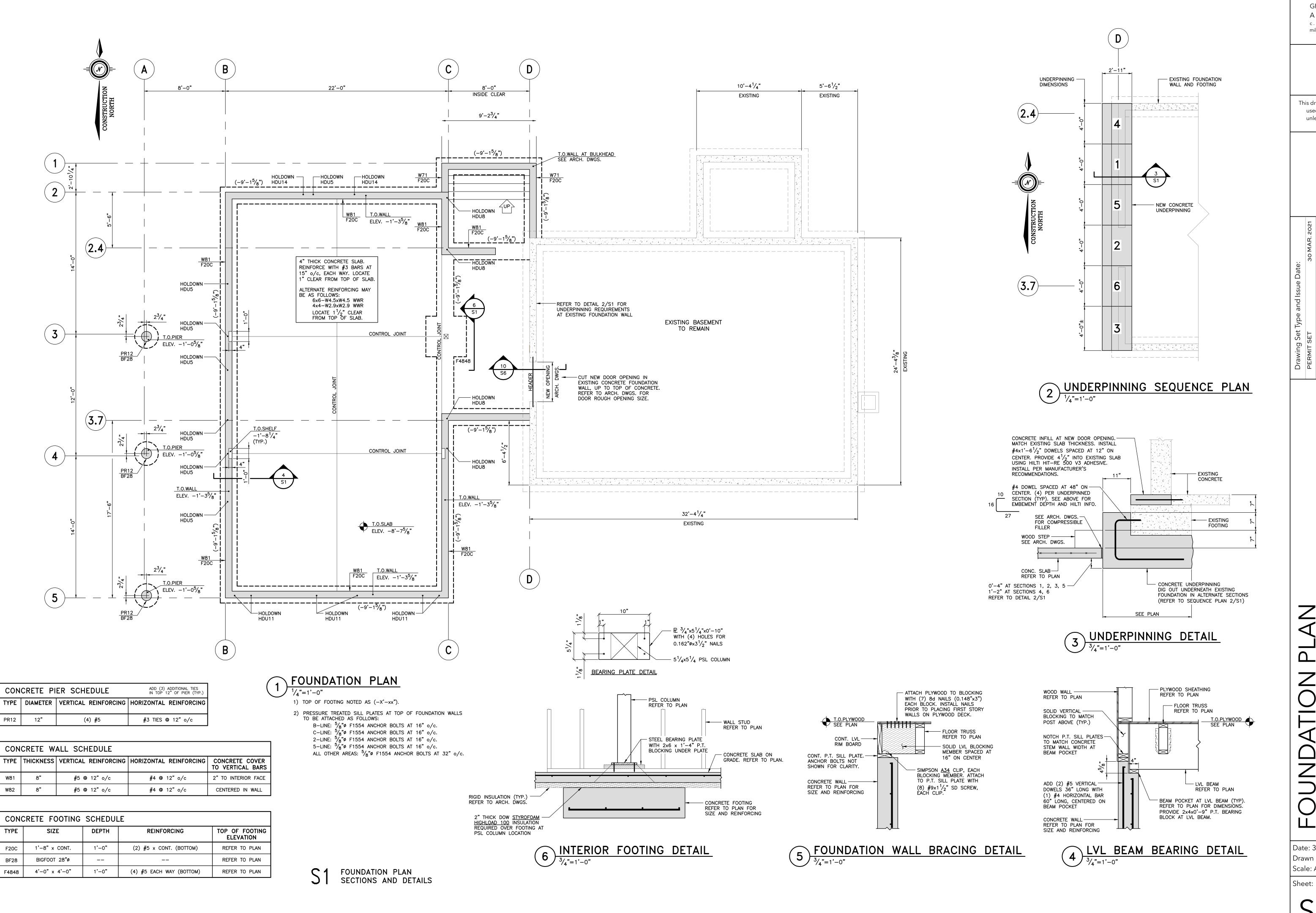
GENERAL NOTES,

TYPICAL DETAILS

DESIGN LOADS AND

* BARS ARE NOT EPOXY COATED TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW THE BAR.

> Date: 3/31/21 Drawn by: GM Scale: As Noted Sheet:



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awing Set Type and Issue Date:

RMIT SET

30 MAR. 2021

TISBURY MA MAP 17. LOT 139

UNDATION PLAN

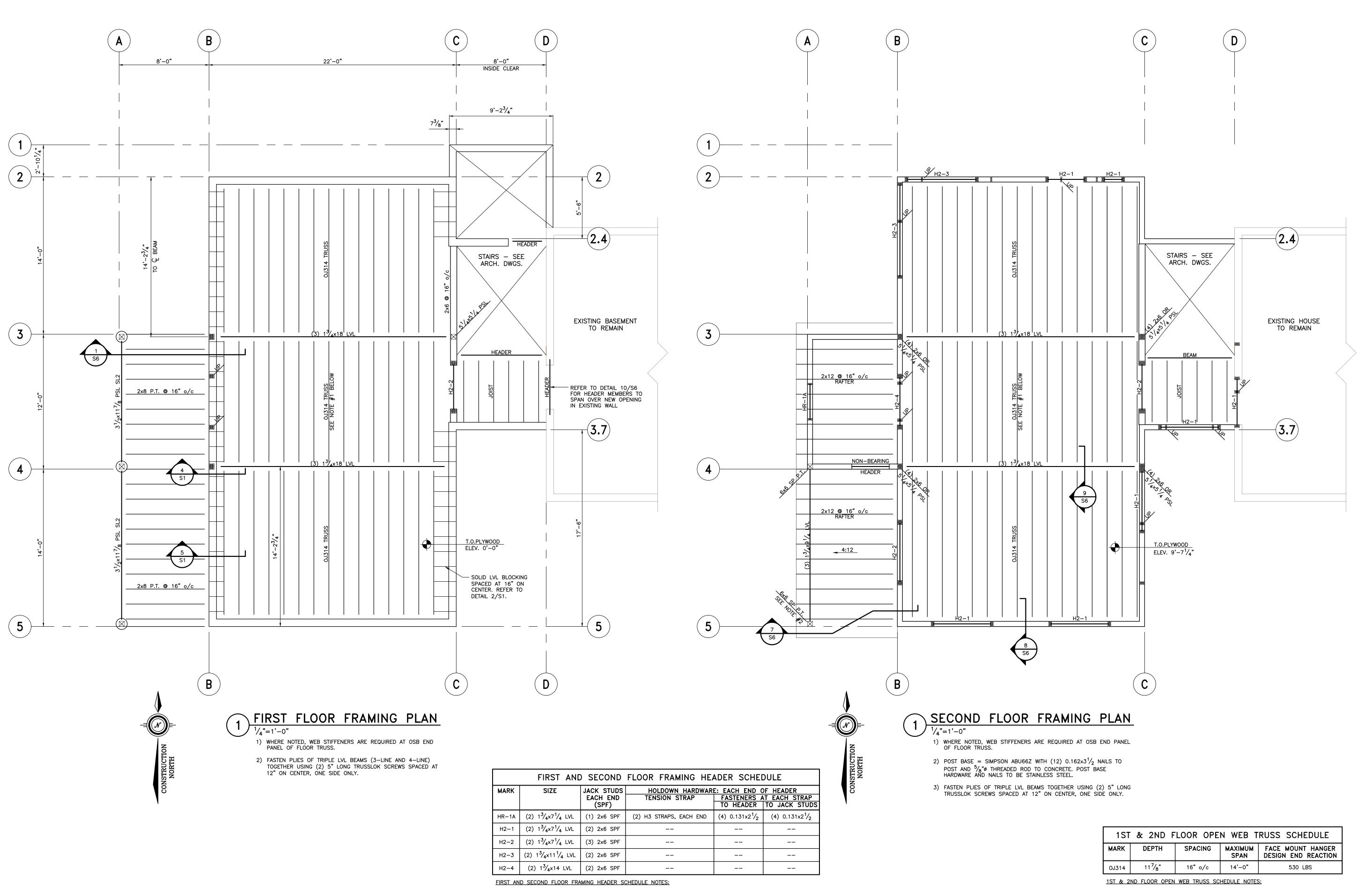
BSTER ADDITION

Date: 3/31/21 Drawn by: GM Scale: As Noted

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S100



1) SPF = SPRUCE-PINE-FIR #1/#2 GRADE.

LVL = MICROLLAM LVL (E = 2.0×10^6 PSI)

2) ALL JACK STUDS AT WINDOW HEADERS SHALL BE LATERALLY BRACED

BY SOLID BLOCKING SPACED VERTICALLY AT 48" ON CENTER (MAX).

S2 FOUNDATION PLAN SECTIONS AND DETAILS

2) SIZE FACE-MOUNT JOIST HANGERS FOR END REACTION SHOWN IN SCHEDULE.

1) OPEN WEB WOOD TRUSSES TO BE MANUFACTURED BY "TRIFORCE OPEN JOIST" OR APPROVED EQUAL.

Date: 3/31/21 Drawn by: GM Scale: As Noted

Sheet:

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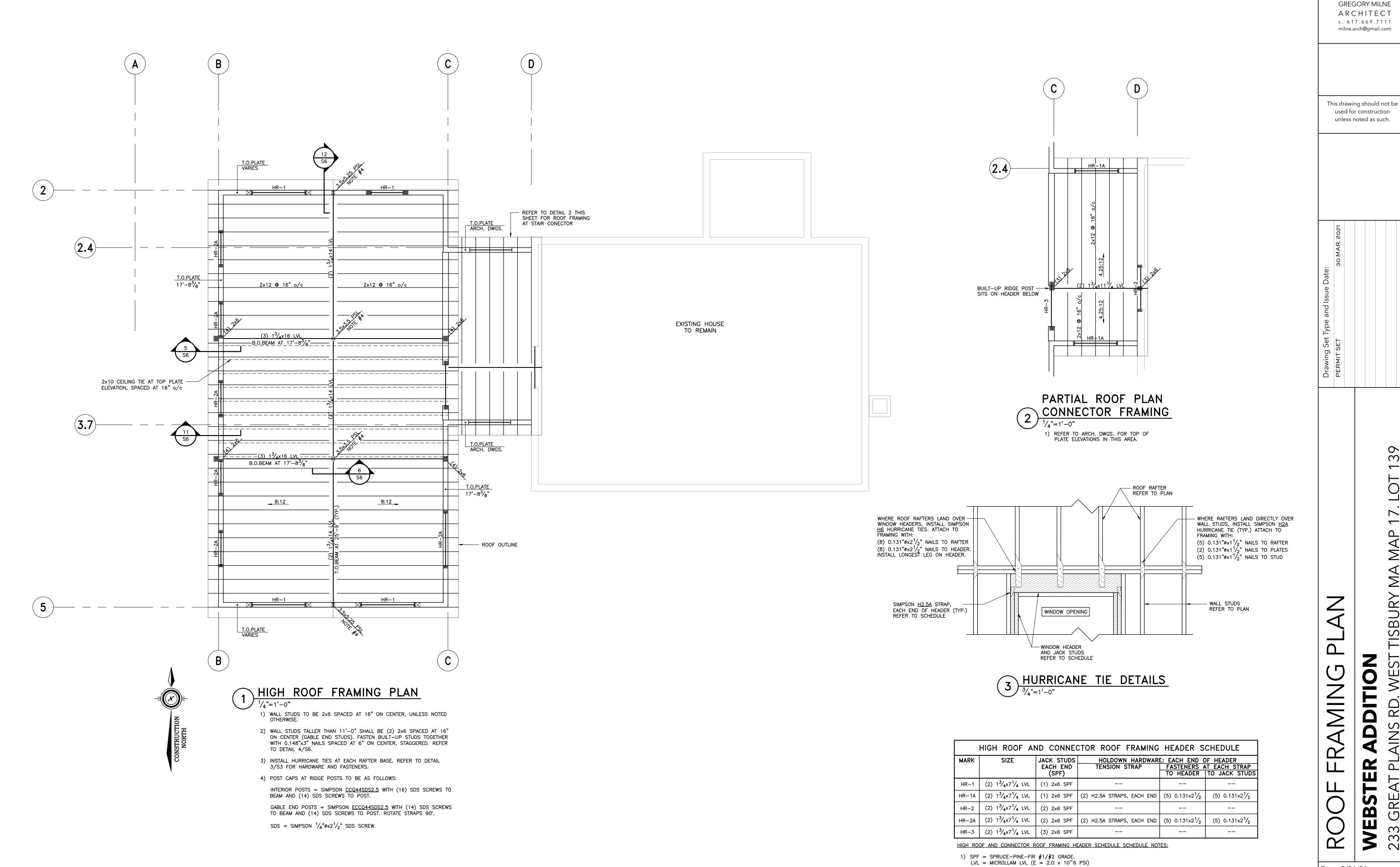
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Date: 3/31/21 Drawn by: GM Scale: As Noted

Sheet:

2) ALL JACK STUDS AT WINDOW HEADERS SHALL BE LATERALLY BRACED BY SOLID

3) REFER TO NAIL SCHEDULE ON SHEET S4 FOR PENNY EQUIVALENT OF NAILS SIZES.

BLOCKING SPACED VERTICALLY AT 48" ON CENTER (MAX).

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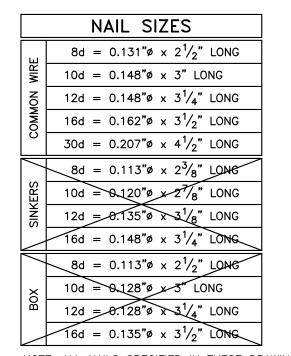
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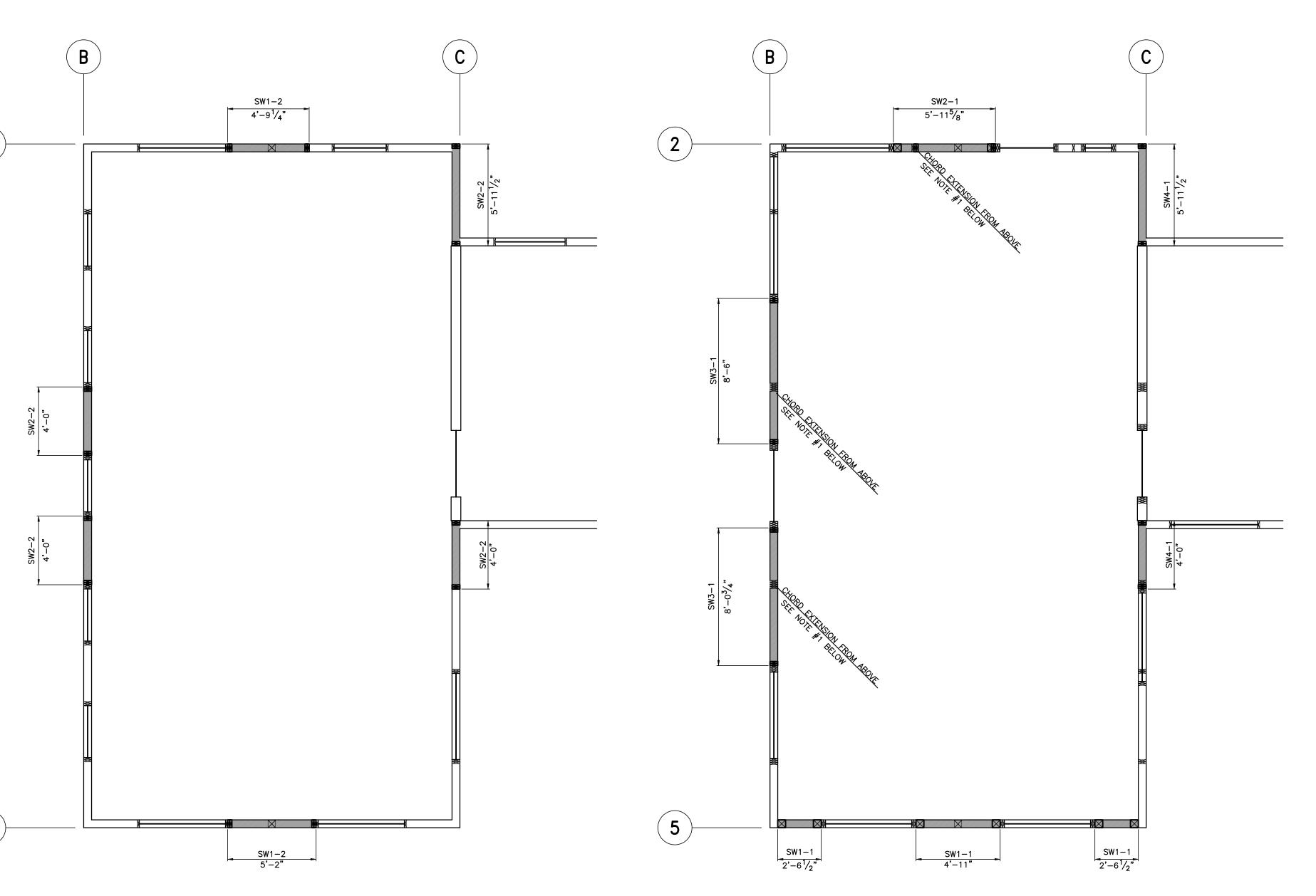
	SHEAR WALL SCHEDULE																
WALL	LEVEL						WALL STUDS	BASE		BASE CONNECTION	N	HEAD	EDGE		HOLDDOWNS		REMARKS
MARK		THICKNESS	WALL FACE APPLIED TO	FASTENER	EDGE	FIELD			SILL PLATE TO RIM	RIM TO PLATE(S)	SILL PLATE TO CONCRETE		CHORD	HOLDOWN	CONN. TO CHORD	ROD	
SW1-2	SECOND	1/2" PLWD	EXTERIOR	8d NAIL	3"o/c	12"o/c	2x6 SPF @ 16" o/c	2x6 SPF	(9) $\frac{1}{4} \times 4^{1} /_{2}$ SDS	(5) LTP4 LATERAL TIES		(2) 2x6 SPF	(2) 2×6	MSTC66 STRAP	(64) 12d NAILS		NOTE 5,8
SW2-2	SECOND	1/2" PLWD	EXTERIOR	8d NAIL	6"o/c	12"o/c	2x6 SPF @ 16" o/c	2x6 SPF	$(4) \frac{1}{4} \times 4 \frac{1}{2} SDS$	(3) LTP4 LATERAL TIES		(2) 2x6 SPF	(2) 2×6	MSTC40 STRAP	(28) 12d NAILS		NOTE 5,8
SW1-1	FIRST	1/2" PLWD	EXTERIOR	8d NAIL	3"o/c	12"o/c	2x6 SPF @ 16" o/c	2x6 SPF	$(10) \frac{1}{4} \times 4 \frac{1}{2} SDS$	(6) LTP4 LATERAL TIES	⁵ / ₈ "ø F1554 BOLTS ◎ 24" o/c	(2) 2x6 SPF	6x6 SYP #1	HDU11-SDS2.5	$(30) SDS^{1}/_{4}"x2^{1}/_{2}"$	1"ø	NOTE 6,8
SW2-1	FIRST	1/2" PLWD	EXTERIOR	10d NAIL	2"o/c	12"o/c	2x6 SPF @ 16" o/c	2x6 SPF	$(19) \frac{1}{4} \times 4 \frac{1}{2} SDS$	(9) LTP4 LATERAL TIES	⁵ / ₈ "ø F1554 BOLTS ◎ 16" o/c	(2) 2x6 SPF	6x6 SYP #1	HDU14-SDS2.5	$(36) SDS^{1}/_{4}"x2^{1}/_{2}"$	1"ø	NOTE 6,7,8
SW3-1	FIRST	1/2" PLWD	EXTERIOR	8d NAIL	6"o/c	12"o/c	2x6 SPF @ 16" o/c	2x6 SPF	$(5) \frac{1}{4} \times 4 \frac{1}{2} SDS$	(3) LTP4 LATERAL TIES	⁵ / ₈ "ø F1554 BOLTS @ 16" o/c	(2) 2x6 SPF	(2) 2x6	HDU5-SDS2.5	$(14) SDS^{1}/_{4}"x2^{1}/_{2}"$	⁵ / ₈ "ø	NOTE 5,8
SW4-1	FIRST	1/2" PLWD	EXTERIOR	8d NAIL	4"o/c	12"o/c	2x6 SPF @ 16" o/c	2x6 SPF	$(8) \frac{1}{4} \times 4 \frac{1}{2} SDS$	(4) LTP4 LATERAL TIES	⁵ / ₈ "ø F1554 BOLTS @ 16" o/c	(2) 2x6 SPF	(2) 2x6	HDU8-SDS2.5	(20) $SDS^{1}/_{4}$ "x2 $^{1}/_{2}$ "	⁷ / ₈ "ø	NOTE 5,8

3W4-1	

- 1) HOLDOWNS BY SIMPSON STRONG-TIE COMPANY.
- 2) A CHORD MEMBER AND HOLDOWN IS REQUIRED AT BOTH ENDS OF THE SHEAR PANEL.
- 3) ALL NAILS SPECIFIED IN SCHEDULE SHALL BE COMMON WIRE NAILS, U.N.O.
- 4) SPF = SPRUCE PINE FIR SYP = SOUTHERN YELLOW PINE
- PL = PARALLAM PSL (PARALLEL STRAND LUMBER)
- PLWD = PLYWOODSDS = SELF DRILLING, SELF TAPPING SCREWS BY SIMPSON STRONG-TIE
- 5) WHERE MULTIPLE 2x MEMBERS ARE SPECIFIED AS EDGE CHORDS, FASTEN PLIES TOGETHER USING (16) 10d NAILS (0.148"x3"), DISTRIBUTED OVER ENTIRE CHORD LENGTH.
- 6) WHERE 1ST LEVEL SILL PLATE ATTACHES TO AN LVL RIM BOARD, THE P.T. SILL PLATE BELOW SHALL ATTACH TO CONCRETE FOUNDATION WALL WITH $^5/_8$ "Ø F1554 ANCHOR BOLTS. REFER TO DETAIL 4 THIS SHEET FOR SPACING.
- 7) WHERE PLYWOOD NAILING IS SPECIFIED AT 2" ON CENTER, MEMBERS AT PLYWOOD EDGES SHALL BE 3" NOMINAL OR (2) 2x6 MEMBERS, FASTENED TOGETHER WITH (6) 10d NAILS (0.148"x3") PER FOOT, ALONG FULL HEIGHT OF BUILT—UP MEMBER.
- 8) <u>LTP4</u> LATERAL TIES TO ATTACH TO FRAMING WITH (12) $8d \times 1^{1}/_{2}$ " NAILS (0.131" $\times 1^{1}/_{2}$ "), TOTAL EACH TIE. ORIENT <u>LTP4</u> IN THE HORIZONTAL DIRECTION AND CENTER VERTICALLY ON THE JOINT BETWEEN RIM BOARD AND PLATE(S).
- 9) EMBEDMENT DEPTH OF HOLDOWN ANCHOR RODS TO BE AS NOTED BELOW. ANCHOR RODS SHALL BE INSTALLED PRIOR TO POURING CONCRETE. FURNISH AND INSTALL NUT AND WASHER AT EMBEDDED END OF ANCHOR ROD.
- $\frac{5}{8}$ "ø F1154 THREADED ROD = 10"
- $\frac{7}{8}$ % F1154 THREADED ROD = 18" 1° ø F1154 THREADED ROD = 20"



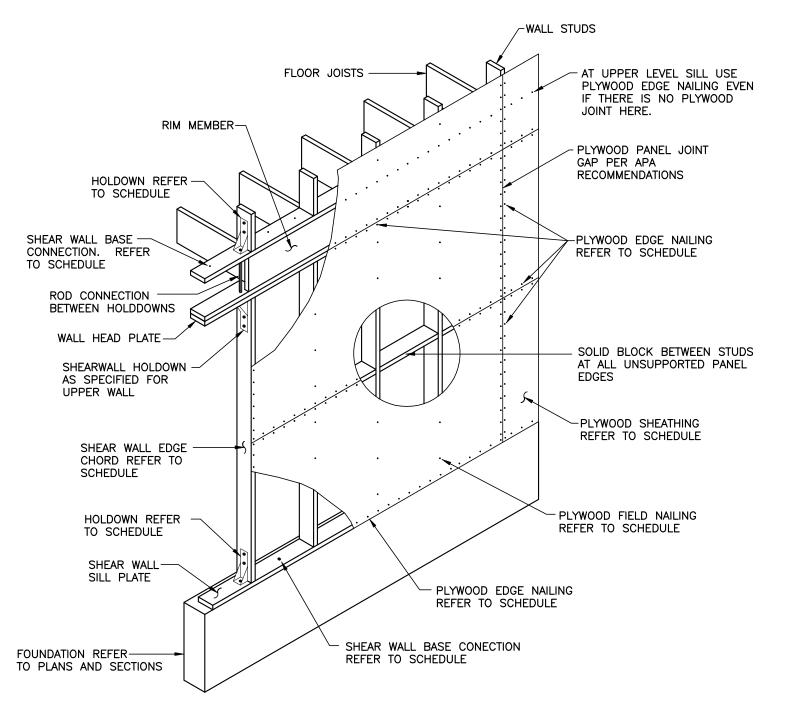
NOTE: ALL NAILS SPECIFIED IN THESE DRAWINGS ARE TO BE COMMON WIRE NAILS, UNLESS SPECIFICALLY NOTED OTHERWISE.



SHEARWALL LAYOUT PLAN 2ND STORY

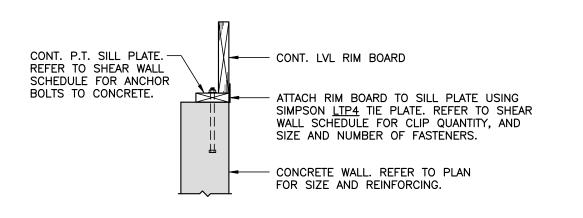
SHEARWALL LAYOUT PLAN

1) WHERE NOTED, INSTALL SIMPSON <u>HDU5-SDS2.5</u> HOLDOWN. ATTACH WITH (14) $^{1}/_{4}$ "x2 $^{1}/_{2}$ " SDS SCREWS TO (2) 2x6 CHORD AND $^{5}/_{8}$ " ϕ F1554 ANCHOR ROD TO CONCRETE. FASTEN 2x6 CHORD MEMBERS TOGETHER WITH (14) 10d NAILS (0.148"x3"), DISTRIBUTED OVER ENTIRE CHORD LENGTH. CHORD TO ALIGN WITH SHEARWALL CHORD FROM UPPER LEVEL.



TYPICAL WOOD SHEAR WALL ISOMETRIC

3/8"=1'-0"



RIM BOARD TO SILL PLATE CONN.

3/4"=1'-0"

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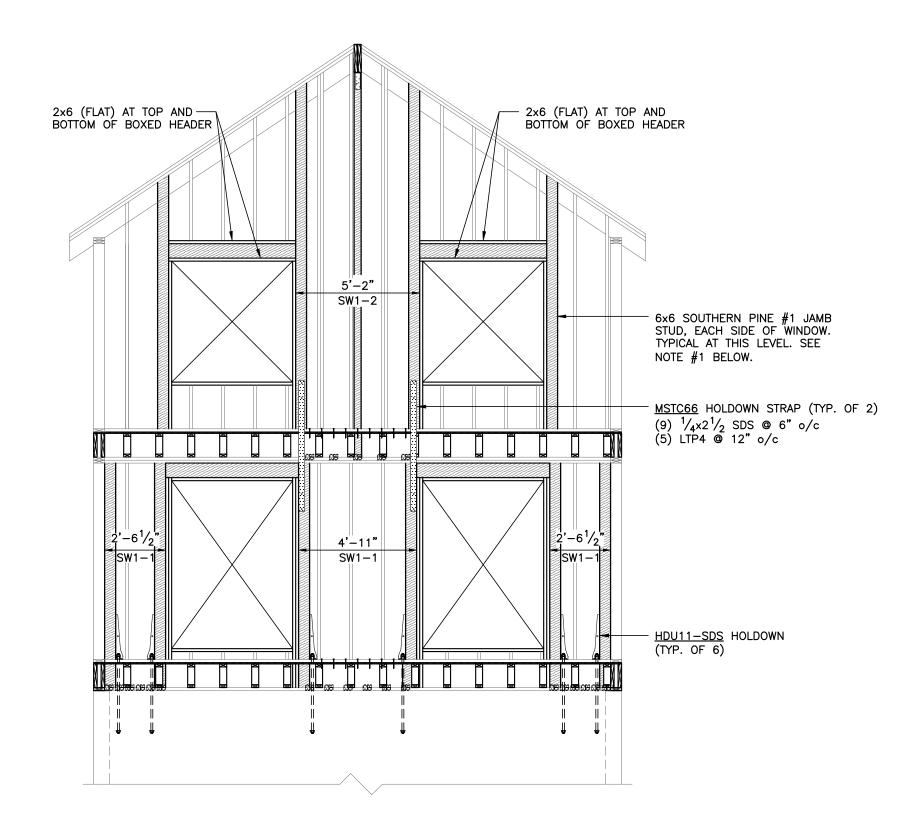
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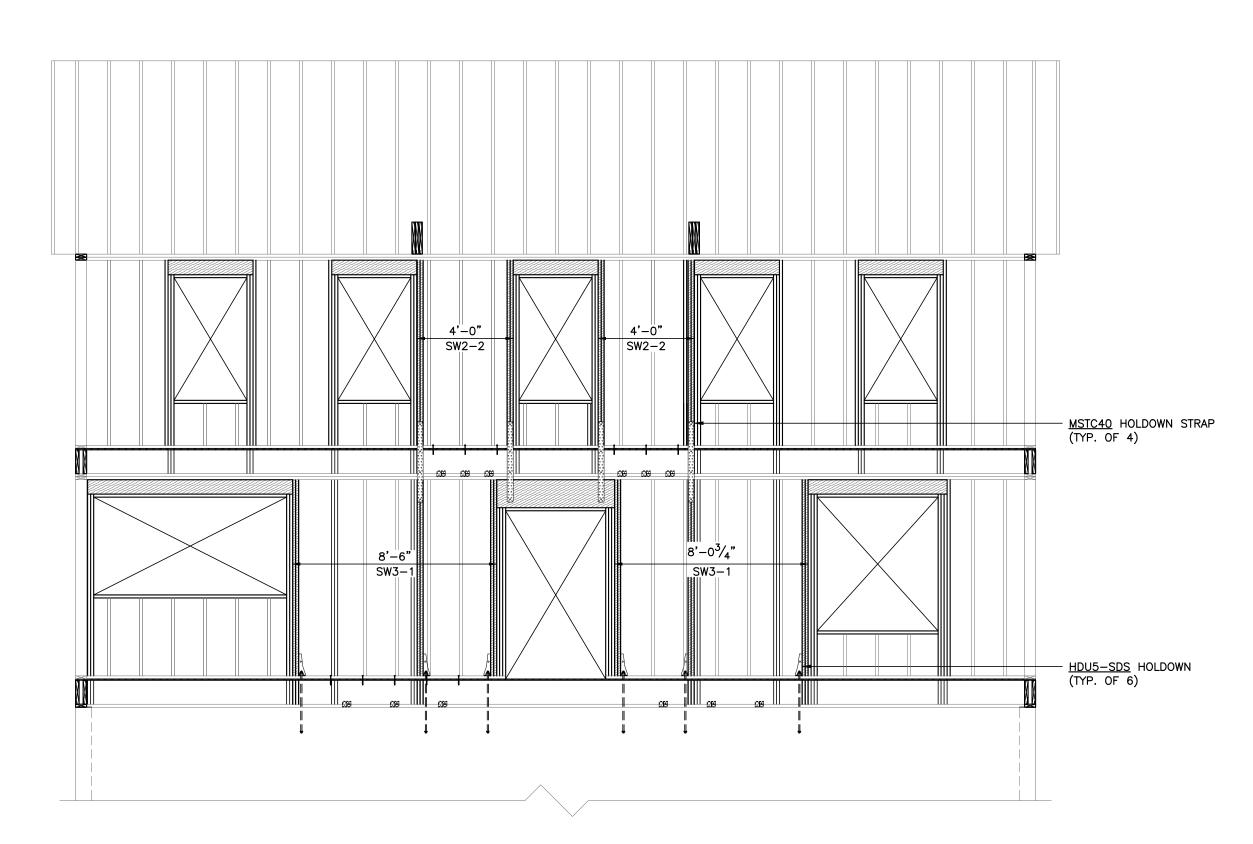
WEB, 233 GR Date: 3/31/21 Drawn by: GM Scale: As Noted

\$4 SHEAR WALL PLANS, SCHEDULE AND DETAILS



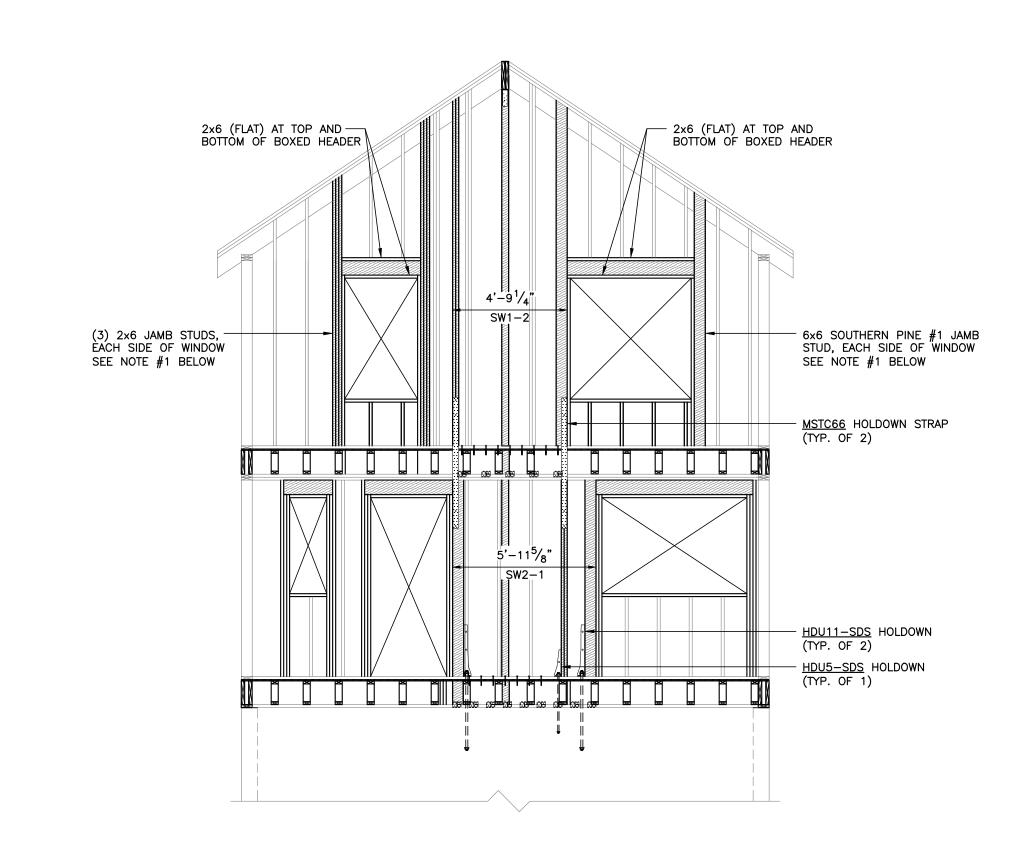
SOUTH SHEAR WALL ELEVATION 1) WHERE NOTED, FASTEN WINDOW JAMB STUD(S) TO SILL PLATE WITH

(4) .0162° $\phi \times 3\frac{1}{2}$ ° TOE-NAILS. USE SAME CONNECTION AT TOP PLATES.



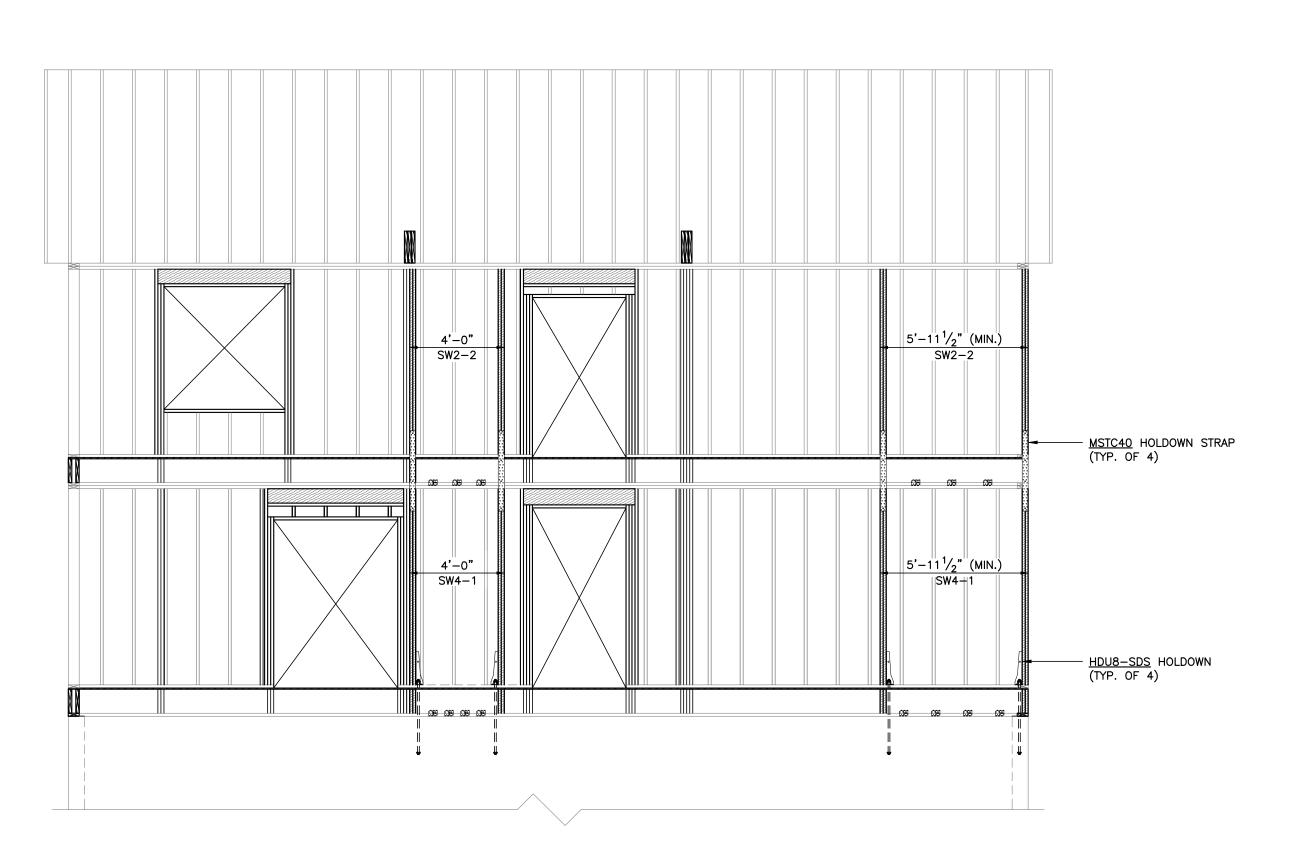
WEST SHEAR WALL ELEVATION

1/4"=1'-0"



NORTH SHEAR WALL ELEVATION

1/4"=1'-0" 1) WHERE NOTED, FASTEN WINDOW JAMB STUD(S) TO SILL PLATE WITH (4) .0162 %x3 $\frac{1}{2}$ " TOE-NAILS. USE SAME CONNECTION AT TOP PLATES.



EAST SHEAR WALL ELEVATION

1/4"=1'-0"

ELEVATIONS WEBSTER, 233 GREAT PLA SHEAR

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17.

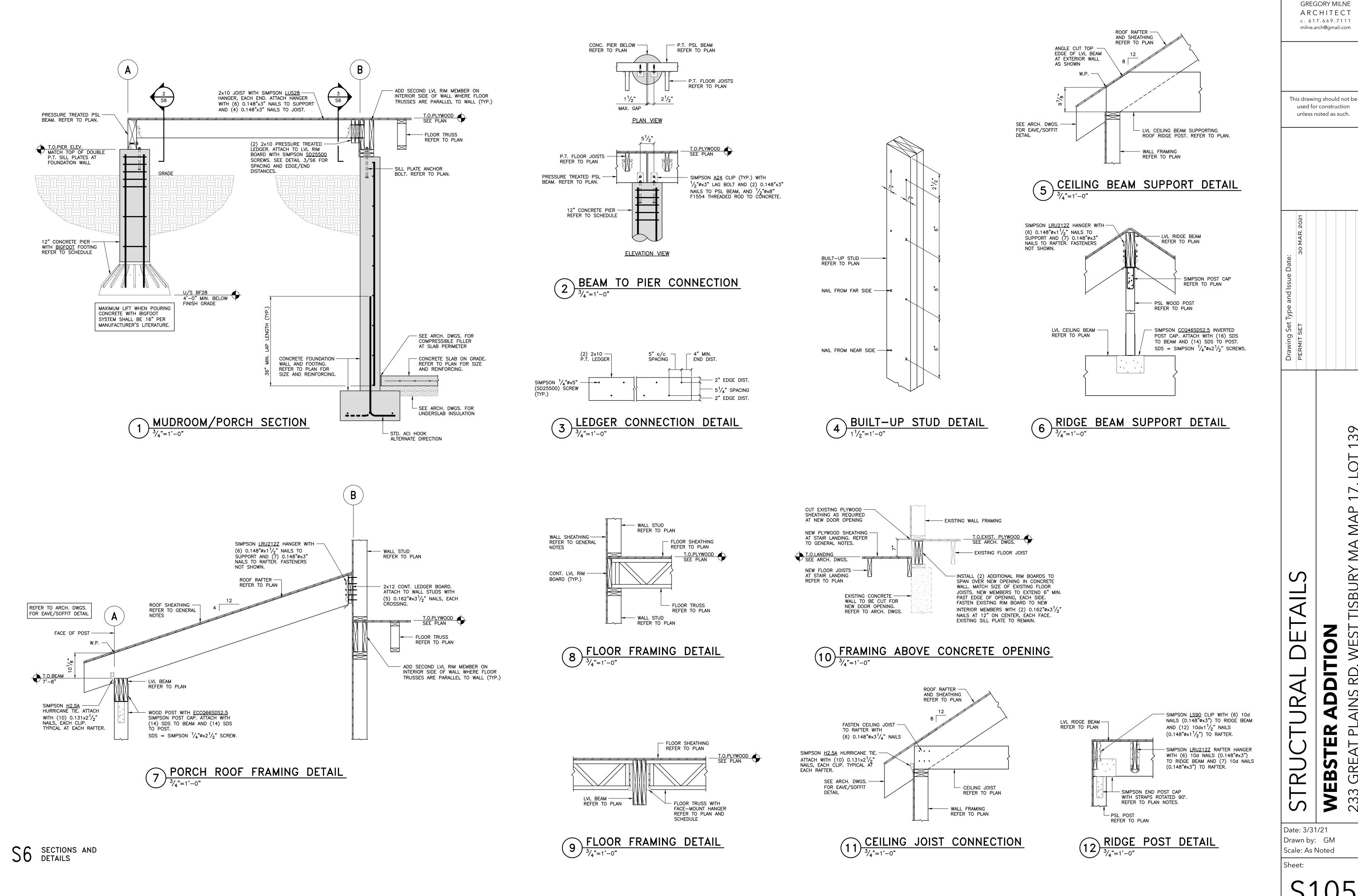
WEST TISBURY MA MAP

AINS RD.

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 $\overline{}$ MAP

TISBURY AINS 2 $\mathbf{\Omega}$

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